

### FOUNDATION NOTES

FOR PILES. SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 65 TONS PER PILE.

DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 108 TONS PER PILE.

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

DRILLED PIERS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 340 TONS/PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 20 TSF.

INSTALL DRILLED PIERS AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 135 FT WITH THE REQUIRED TIP RESISTANCE.

PERMANENT STEEL CASINGS ARE REQUIRED FOR DRILLED PIERS AT BENT NO.1.DO NOT EXTEND PERMANENT CASINGS BELOW ELEVATION 160 FT WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

INSTALL PERMANENT STEEL CASINGS AT BENT NO.1 BY VIBRATING, SCREWING OR DRIVING PERMANENT CASINGS BEFORE EXCAVATING OR DISTURBING ANY MATERIAL BELOW ELEVATION 175 FT.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 IS 170 FT.THE SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

TO VERIFY BEARING STRATA, STANDARD PENETRATION TESTING (SPT) IS REQUIRED FOR DRILLED PIERS AT BENT NO.1 AT THE FINAL TIP ELEVATION INDICATED. FOR SPT TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

DRILLED PIERS AT BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 435 TONS/PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 20 TSF.

INSTALL DRILLED PIERS AT BENT NO.2 TO A TIP ELEVATION NO HIGHER THAN 156 FT WITH THE REQUIRED TIP RESISTANCE AND A PENETRATION OF AT LEAST 21 FT INTO WEATHERED ROCK.

PERMANENT STEEL CASINGS ARE REQUIRED FOR DRILLED PIERS AT BENT NO.2.DO NOT EXTEND PERMANENT CASINGS BELOW ELEVATION 177 FT WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.2 IS 174 FT. THE SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

TO VERIFY BEARING STRATA, STANDARD PENETRATION TESTING (SPT) IS REQUIRED FOR DRILLED PIERS AT BENT NO. 2. PERFORM SPTS AT ELEVATION 177.3 FT TO VERIFY TOP OF PARTIALLY WEATHERED ROCK AND AGAIN AT THE FINAL TIP ELEVATION INDICATED. FOR SPT TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

DRILLED PIER EXCAVATIONS AT BENTS NOS.1 AND 2 WILL EXTEND INTO MATERIAL THAT DETERIORATES WHEN EXPOSED TO THE ELEMENTS. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE AND PLACE CONCRETE IMMEDIATELY AFTER THE EXCAVATION IS COMPLETED.

SID INSPECTIONS ARE REQUIRED FOR DRILLED PIERS AT BENTS NOS.1 AND 2. FOR SID INSPECTIONS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR THE DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING, FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.

DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.

### GENERAL NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE "STANDARD NOTES" SHEET.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THE EXISTING STRUCTURE CONSISTING OF (1) 41'-2", (2) 50'-1", (1) 40'-1", AND (1) 41'-2" SPANS WITH STEEL PLANK DECK ON STEEL I-BEAMS WITH A CLEAR ROADWAY OF 24'-1" AND SUPPORTED BY CONCRETE ABUTMENTS AND INTERIOR BENTS SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. FOR REMOVAL OF EXISTING STRUCTURE, SEE SPECIAL PROVISIONS

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 15+17.00 -L-".

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA (ON SHEET 1 OF 2) SHALL BE EXCAVATED FOR A DISTANCE FROM THE CENTERLINE OF ROADWAY OF 33'± (LEFT) AND 31'± (RIGHT) AT END BENT 1, AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE, SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES".

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK. SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

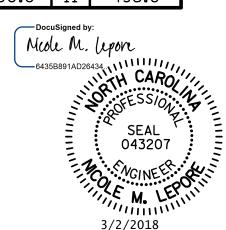
FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

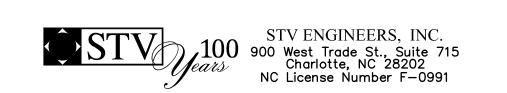
ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

	TOTAL BILL OF MATERIAL												
	REMOVAL OF EXISTING STRUCTURE AT STA.15+17.00 -L-	ASBESTOS ASSESSMENT	3'-0"Ø DRILLED PIERS IN SOIL	3'-0"Ø DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 3'-O" DRILLED PIER	SID INSPECTIONS	SPT TESTING	CSL TESTING (IF REQUIRED)	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL
	LUMP SUM	LUMP SUM	LIN.FT.	LIN. FT.	LIN.FT.	EA.	EA.	EA.	LUMP SUM	CU. YD.	LUMP SUM	LBS.	LBS.
SUPERSTRUCTURE	LUMP SUM										LUMP SUM		
END BENT 1										21.6		2,636	
BENT 1			150.0	9.0	84.0	3	3			22.9		17,507	3,298
BENT 2			61.0	32.0	29.4	3	6			21.3		12,244	2,247
END BENT 2										25.6		<b>3,</b> 576	
TOTAL	LUMP SUM	LUMP SUM	211.0	41.0	113.4	6	9	1	LUMP SUM	91.4	LUMP SUM	35,963	5,545

TOTAL BILL OF MATERIAL (CONT'D)											
	PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES	HP S	12X53 STEEL PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE CC	STRESSED NCRETE	PRE CC	O"X 1'-9" STRESSED NCRETE ED SLABS
	EA.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.	NO.	LIN.FT.
SUPERSTRUCTURE				450.0			LUMP SUM	11	1980.0	11	495.0
END BENT 1	7	7	455.0		130	145					
BENT 1											
BENT 2											
END BENT 2	7	7	175.0		150	165					
TOTAL	14	14	630.0	450.0	280	310	LUMP SUM	11	1980.0	11	495.0





DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT NO	). <u> </u>	-5817
ANS		COUNTY
STATION:	15+17.0	00 -L-

SHEET 2 OF 2

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE ON SR 1634
(GRASSY ISLAND ROAD)
OVER BROWN CREEK
BETWEEN NC109 AND NC52

		SHEET NO.							
) <b>.</b> (	BY:	DATE:	NO.	BY:	DATE:	S-2			
			<b>જી</b>			TOTAL SHEETS			
2			4			26			

ASSEMBLED BY: LEM DATE: 1-18

CHECKED BY: MLO DATE: 1-18

DESIGN ENGINEER OF RECORD: NML DATE: 3-18

DRAWN BY: CVC 6/IO
CHECKED BY: DNS 6/IO

TNAGT5A

TNAGT5B

45.000

45.000

1.033

1.009 | 45.408

#### LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE MOMENT SHEAR MOMENT LOCATIO DISTRIBUTION FACTORS (DF) LIVELOAD FACTORS DISTRIBU<sup>-</sup> FACTORS ( DISTRIE FACTORS RATING GIRDER GIRDER DIST/ LEFT SPAN SPAN CONTI GIRD DIS LEF SPAI 0.277 1.088 0.277 1.34 0.539 1.23 0.80 1.09 HL-93(Inv)1.75 45′ EL 22 45′ EL 2.2 EL 22 N/A 0.277 0.539 1.59 HL-93(0pr) N/A 1.590 1.35 1.74 45′ EL 22 45′ EL 2.2 N/A DESIGN LOAD 36.000 1.336 1.75 0.277 1.65 45′ EL 22 0.539 1.45 45′ EL 2.2 0.80 0.277 1.34 HS-20(Inv)45′ EL RATING 36.000 1.882 1.35 0.277 2.14 22 0.539 1.88 2.2 HS-20(0pr) 45′ EL 45′ EL N/A 0.539 0.80 0.277 2.61 13.500 2.611 35.252 0.277 4.02 22 45′ EL 2.2 SNSH 45′ EL 4.01 45′ EL 22 20.000 2.108 42.166 0.277 3.25 22 0.539 2.94 0.80 0.277 2.11 SNGARBS2 45′ EL 45′ EL 2.2 45′ 22 EL 0.277 3.15 17.6 0.539 2.77 0.80 0.277 2.07 SNAGRIS2 22.000 2.067 45.466 45′ EL 45′ EL 2.2 45′ EL 27.250 1.304 35.527 0.277 2.01 45′ EL 22 0.539 2.01 45′ EL 2.2 0.80 0.277 1.30 45′ 22 SNCOTTS3 EL 0.277 1.15 34.925 1.150 0.277 1.77 22 0.539 1.74 2.2 0.80 SNAGGRS4 45′ EL 45′ EL 45′ EL 22 40.181 0.277 35.550 1.121 0.277 22 0.539 1.79 0.80 1.12 SNS5A 39.841 1.73 45′ EL 45′ EL 2.2 45′ EL 22 1.4 0.277 1.63 0.539 1.67 0.80 0.277 1.06 22 45′ EL 2.2 45′ SNS6A 39.950 1.056 42.175 45′ EL EL 22 42.000 1.006 42.268 0.277 1.55 0.539 1.68 0.80 0.277 1.01 SNS7B 45′ EL 22 45′ EL 2.2 45′ EL 22 LEGAL LOAD 0.277 TNAGRIT3 33.000 42.759 0.277 2.00 45′ EL 22 0.539 1.96 45′ EL 2.2 0.80 1.30 45′ 22 EL RATING 33.075 1.309 43.305 0.277 2.02 22 0.539 1.88 45′ 2.2 0.80 0.277 TNT4A 45′ EL EL 1.31 45′ EL 22 1.4 0.277 1.69 0.539 1.83 0.80 0.277 1.10 45′ EL 22 45′ EL 2.2 45′ TNT6A 41.600 1.099 45.712 EL 22 42.000 0.277 0.539 1.69 0.80 0.277 1.12 45′ EL 22 45′ EL 2.2 TNT7A 1.120 47.043 1.73 45′ EL 22 42.000 48.975 0.277 1.80 0.539 1.61 0.80 0.277 TNT7B 1.166 45′ EL 22 45′ EL 2.2 1.17 45′ EL 22 43.000 47.757 0.277 1.71 45′ EL 22 0.539 1.55 45′ EL 2.2 0.80 0.277 1.11 45′ EL 22 TNAGRIT4

0.539

0.539

1.59

1.47

45′

0.80

0.80

0.277

0.277

1.03

1.01

45′

EL

EL

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LIMIT STATE YDC YDW

STRENGTH I 1.25 1.50

SERVICE III 1.00 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

### COMMENTS:

Ι.

۷.

4.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

 $\sqrt{3}$  LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-5817

ANSON COUNTY

STATION: 15+17.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

LRFR SUMMARY FOR

45' CORED SLAB UNIT

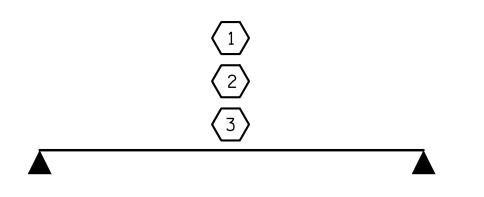
90° SKEW

(NON-INTERSTATE TRAFFIC)

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-3

1 3 TOTAL SHEETS
26



0.277

0.277

1.4

1.59

1.56

LRFR SUMMARY
FOR SPAN A

STV ENGINEERS, INC.

900 West Trade St., Suite 715
Charlotte, NC 28202
NC License Number F-0991

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Mode M. lepone

LEM ASSEMBLED BY : \_ DATE : <u>1-18</u> MLO \_\_ DATE : <u>1-18</u> CHECKED BY : \_\_\_\_ DESIGN ENGINEER OF RECORD : NML DATE : 3-18 DRAWN BY: TMG II/II

CHECKED BY : AAC II/II

43.000

45.000

45.000

TNAGRIT4

TNAGT5A

TNAGT5B

1.258

54.101

1.178 | 53.027

#### LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE MOMENT SHEAR MOMENT DISTRIBUTION FACTORS (DF) LIVELOAD FACTORS DISTRIBU<sup>.</sup> FACTORS ( GIRDER GIRDER DIST/ LEFT SPAN CONTI DISTI FACT GIRD DIS LEF SPA B OR 0.272 0.272 1.47 44.25 0.493 1.26 0.80 HL-93(Inv)1.109 1.75 B OR C EL 4.425 1.11 B OR C 44.25 N/A EL EL HL-93(0pr) N/A 1.633 1.35 1.90 B OR C EL 44.25 0.493 1.63 B OR C EL 4.425 N/A DESIGN LOAD 36.000 1.507 54.255 1.75 0.272 1.99 B OR C EL 44.25 0.493 1.65 B OR C EL 4.425 0.80 0.272 1.51 B OR C 44.25 HS-20(Inv)EL RATING 2.59 B OR C 36.000 77.039 1.35 0.272 44.25 0.493 2.14 B OR C 4.425 HS-20(0pr) EL EL N/A 0.272 3.52 44.25 B OR 4.425 44.25 13.500 3.519 47.501 0.272 0.493 0.80 SNSH B OR C 5.05 EL B OR C EL EL 2.572 0.272 3.55 0.80 0.272 2.57 SNGARBS2 20.000 51.43 4.25 B OR C 44.25 0.493 B OR EL 4.425 B OR C 44.25 EL EL 0.272 44.25 3.27 B OR C 4.425 0.80 0.272 2.41 B OR C SNAGRIS2 22.000 2.415 53.122 4.00 B OR C EL 0.493 EL EL 44.25 B OR C 27.250 1.749 47.674 0.272 2.89 B OR C EL 44.25 0.493 2.52 EL 4.425 0.80 0.272 1.75 B OR C 44.25 SNCOTTS3 0.272 34.925 1.443 50.381 0.272 2.39 B OR C 44.25 0.493 2.06 B OR 4.425 0.80 1.44 44.25 SNAGGRS4 EL B OR C EL EL B OR C 0.272 50.195 0.272 0.80 44.25 SNS5A 35.550 2.34 B OR C EL 44.25 0.493 2.07 EL 4.425 1.41 B OR C EL 1.412 0.272 2.13 B OR C 44.25 1.88 4.425 0.80 0.272 1.29 0.493 B OR C EL B OR C 44.25 SNS6A 39.950 1.287 51.435 EL EL 42.000 1.226 51.483 0.272 2.03 B OR C 44.25 1.83 4.425 0.80 0.272 1.23 SNS7B EL 0.493 B OR C EL B OR C 44.25 LEGAL LOAD 0.272 1.57 44.25 TNAGRIT3 33.000 1.568 0.272 2.59 B OR C 0.493 2.24 B OR EL 4.425 0.80 B OR C 44.25 51.733 EL EL RATING 33.075 1.572 52.007 0.272 44.25 B OR C 4.425 0.80 0.272 1.57 TNT4A 2.60 B OR C 0.493 2.2 EL B OR C 44.25 1.4 EL EL 2.11 | B OR C 53.17 0.272 44.25 1.92 4.425 0.80 0.272 1.28 B OR C B OR C EL 44.25 TNT6A 41.600 1.278 EL 0.493 EL 42.000 1.281 53.782 0.272 2.12 B OR C 44.25 1.89 4.425 0.80 0.272 1.28 0.493 B OR C EL B OR C 44.25 TNT7A EL EL 42.000 1.315 55.229 0.272 2.18 B OR C 44.25 0.493 1.79 B OR C 4.425 0.80 0.272 1.31 B OR C TNT7B EL EL EL 44.25

44.25

EL

0.493

0.493

0.493

B OR C

B OR C

1.66 | B OR C | EL

EL

4.425

4.425

4.425

1.74

LOAD FACTORS:

LIMIT STATE  $\gamma_{\sf DC}$ DESIGN 1.25 | 1.50 STRENGTH I RATING **FACTORS** SERVICE III | 1.00 | 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

### COMMENTS:

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

 $\langle 3 \rangle$  LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

B-5817 PROJECT NO. \_\_\_ ANSON

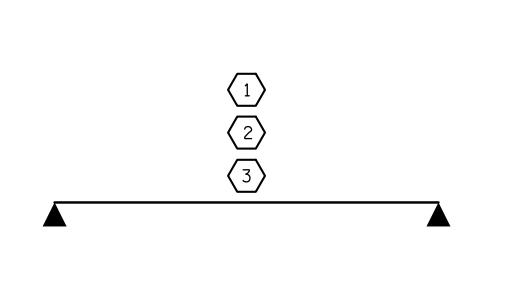
15+17.00 -L-STATION:

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

COUNTY

STANDARD LRFR SUMMARY FOR 90' BOX BEAM UNIT 90° SKEW (NON-INTERSTATE TRAFFIC)

	SHEET NO.				
D. BY:	DATE:	NO.	BY:	DATE:	S-4
]		3			TOTAL SHEETS
2		4			26



0.272

0.272

2.08 B OR C

1.95 B OR C

B OR C

LRFR SUMMARY

FOR SPANS B & C

STV ENGINEERS, INC.
900 West Trade St., Suite 715
Charlotte, NC 28202
NC License Number F-0991

0.272

0.272

0.272

1.26

1.19

B OR C

B OR C

1.18 B OR C

44.25

44.25

EL

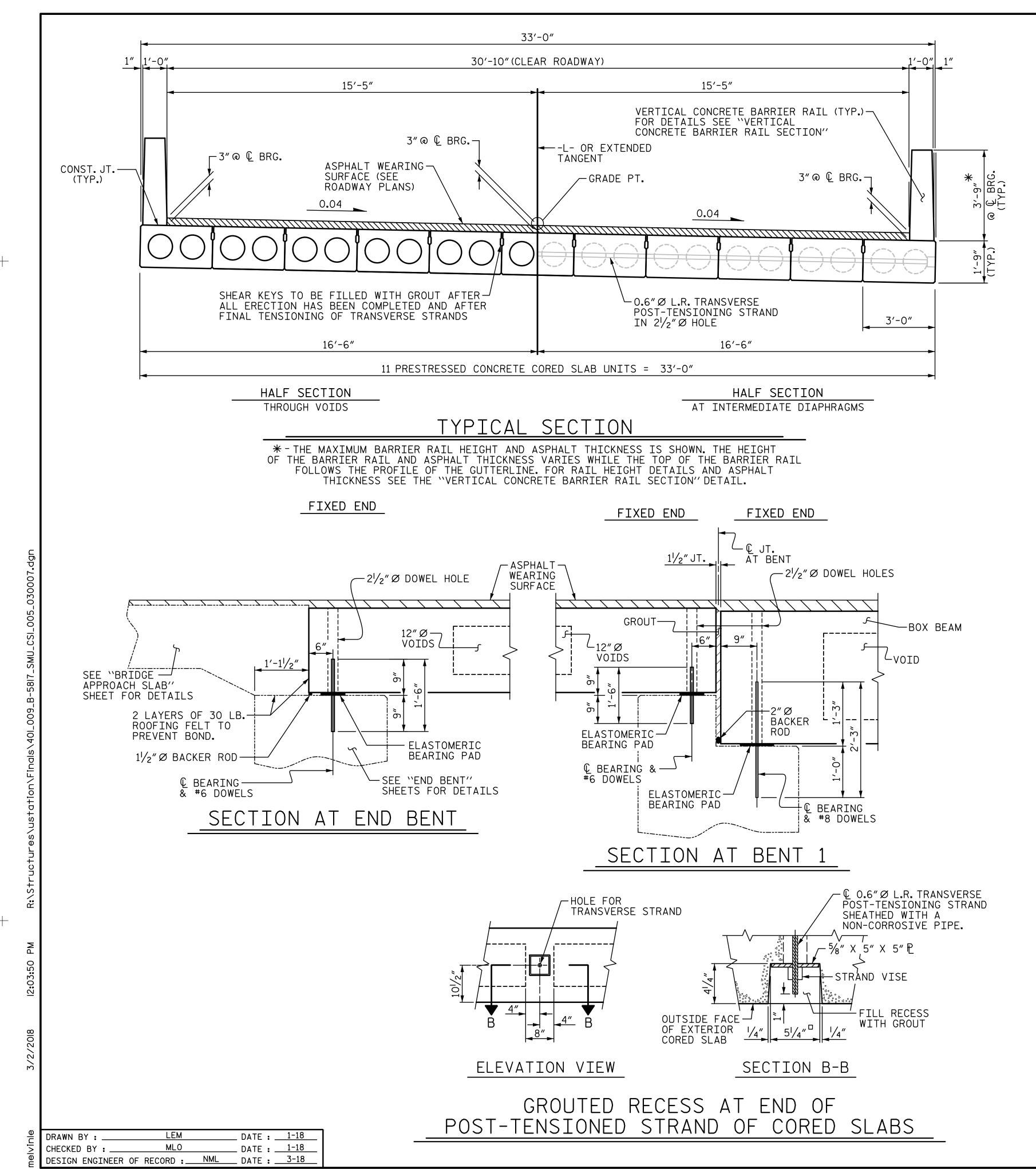
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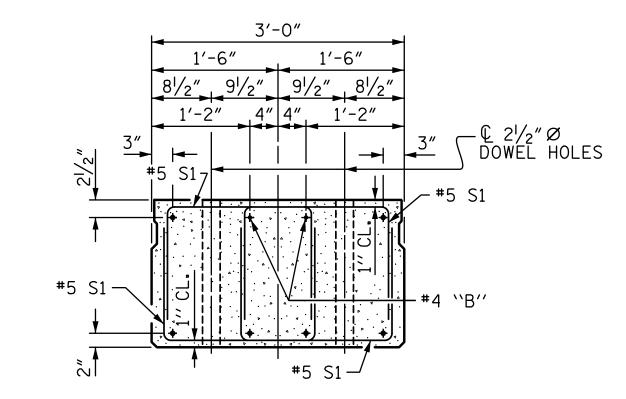
0.80

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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

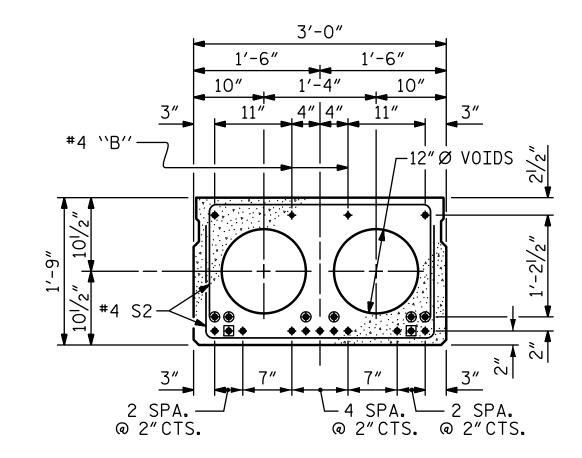
Mole M. Upon





## END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS
AND LOCATION OF DOWEL HOLES.
(STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB
UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.



## INTERIOR SLAB SECTION (13 STRANDS REQUIRED)

## 0.6" Ø LOW RELAXATION STRAND LAYOUT

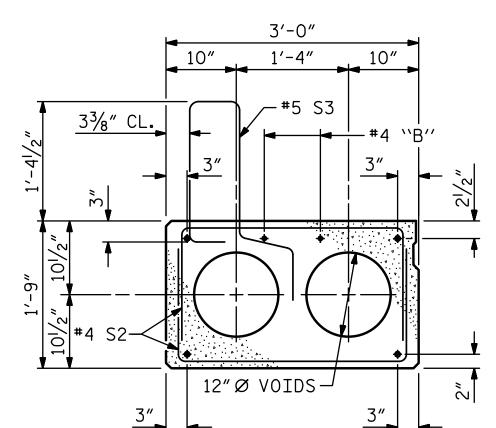
- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-O"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS.
  THESE STRANDS ARE NOT REQUIRED. IF THE
  FABRICATOR CHOOSES TO INCLUDE THESE STRANDS
  IN THE CORED SLAB UNIT, THE STRANDS SHALL
  BE DEBONDED FOR THE FULL LENGTH OF THE UNIT
  AT NO ADDITIONAL COST. SEE STANDARD
  SPECIFICATIONS, ARTICLE 1078-7.

## DEBONDING LEGEND



STV ENGINEERS, INC.
900 West Trade St., Suite 715
Charlotte, NC 28202
NC License Number F-0991

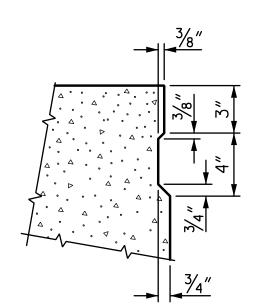
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



EXT. SLAB SECTION

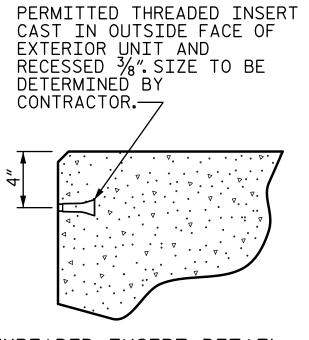
(FOR PRESTRESSED STRAND LAYOUT, SEE

INTERIOR SLAB SECTION.)



SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.



THREADED INSERT DETAIL

PROJECT NO. B-5817

ANSON

STATION: 15+17.00 -L-

SHEET 1 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

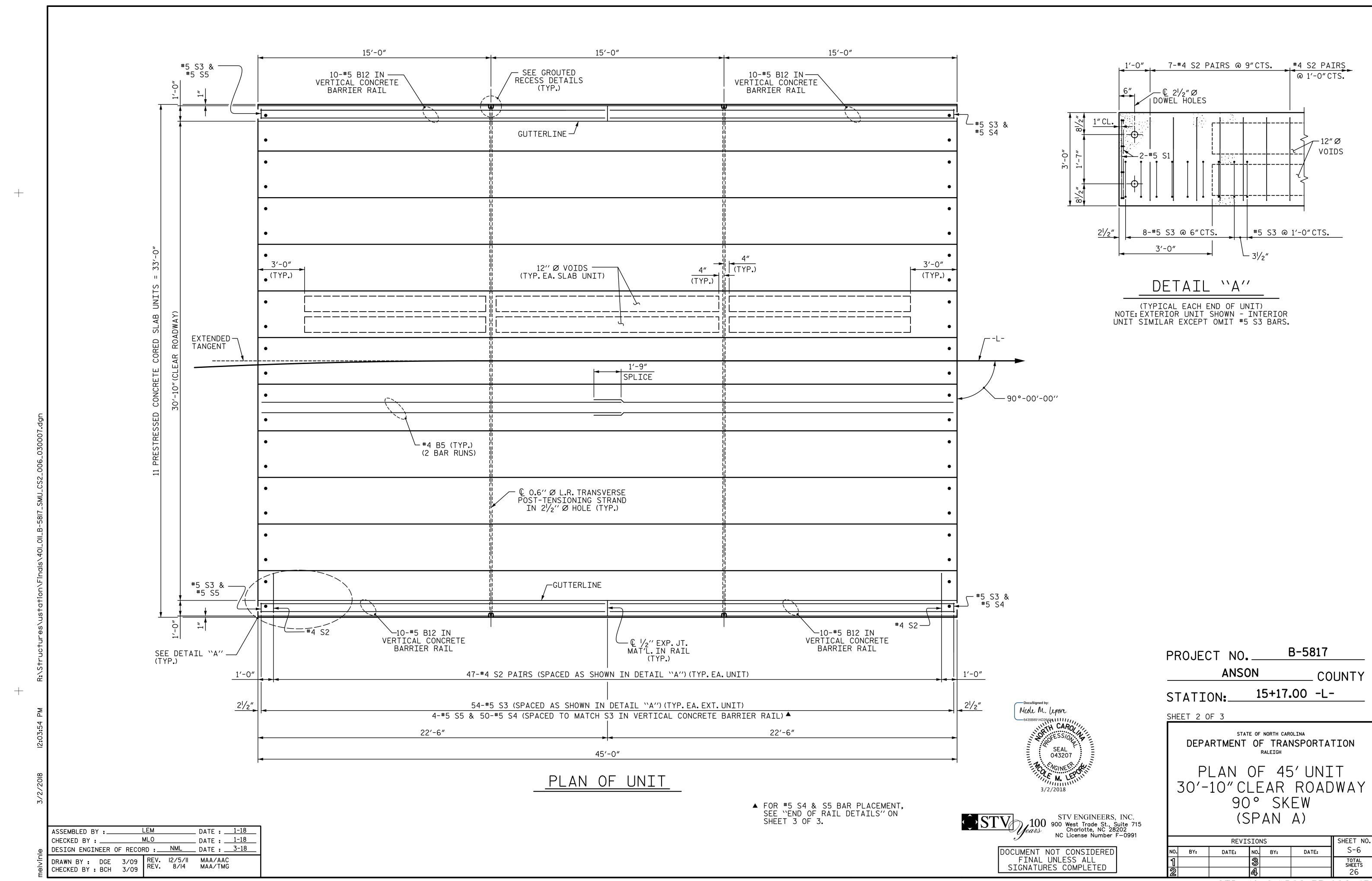
COUNTY

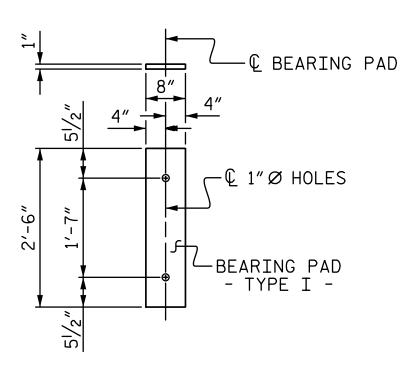
3'-0" X 1'-9"
PRESTRESSED CONCRETE
CORED SLAB UNIT
(SPAN A)

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-5

1 3 TOTAL SHEETS
26





FIXED END (TYPE I - 22 REQ'D)

## ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

GUTTERLINE	ASPHALT THICKNESS &	RAIL HEIGHT
	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
45' UNITS	15/8″	3′-75⁄8″

1'-0"

—#5 S4

(TYP.)

23/8" CL.

-#5 S3 \(\frac{1}{2}\)

2"CL. MIN.

BILL OF MATERIAL FOR ONE 45' CORED SLAB UNIT									
				EXTERI	OR UNIT	INTERI	OR UNIT		
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT		
B5	4	#4	STR	23'-3"	62	23'-3"	62		
S1	8	#5	3	4'-3"	35	4′-3″	35		
S2	94	#4	3	5′-4″	335	5′-4″	335		
* S3	54	#5	1	5′-7″	314				
REINF	ORCING S	STEEL	LBS	S	432		432		
	XY COATE			•	74.4				
	VFORCING				314				
5000 P.S.I. CONCRETE CU. YDS. 6.5							6.5		
0.6"Ø	L.R. STR	<u>ANDS</u>	No.	).	13		13		

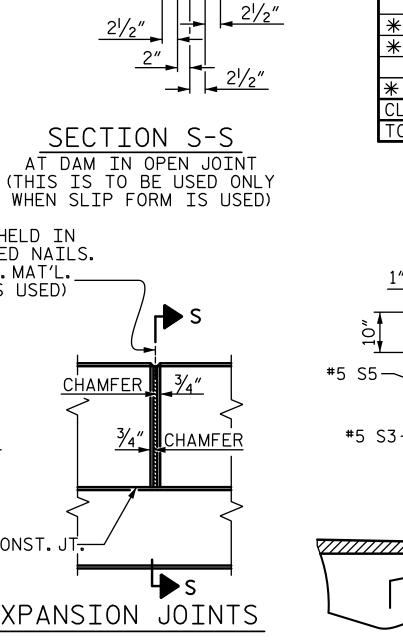
DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 1'-9"
45' CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	7⁄8″ Å
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	l∕8″ <b>†</b>
FINAL CAMBER	3⁄4″ ♦
** INCLUDES FUTURE WEARING SURF	ACE

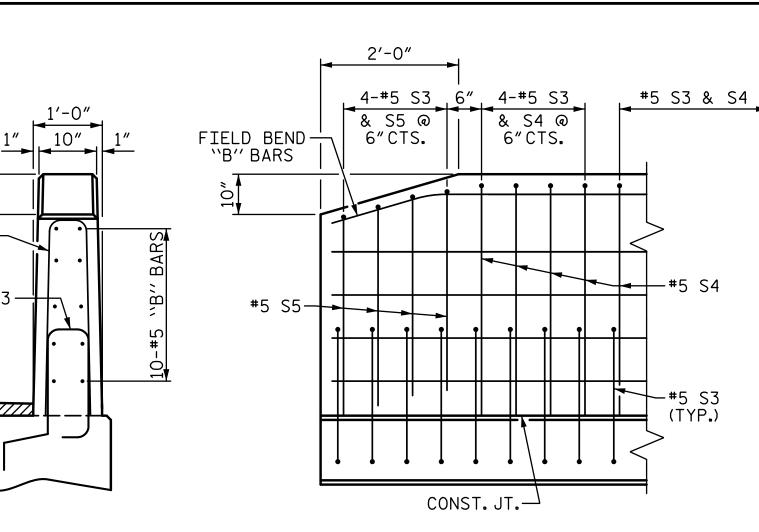
BAR TYPES 63/4" MIN 2'-8'' ALL BAR DIMENSIONS ARE OUT TO OUT

CORED			
	NUMBER	LENGTH	TOTAL LENGTH
45' UNIT			
EXTERIOR C.S.	2	45'-0"	90'-0"
INTERIOR C.S.	9	45'-0"	405′-0″
TOTAL	11		495′-0″

GRADE 270 STRANDS				
	0.6″Ø L.R.			
AREA (SQUARE INCHES)	0.217			
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600			
APPLIED PRESTRESS	43,950			

#### BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL BAR | BARS PER PAIR OF EXTERIOR UNITS | TOTAL NO. | SIZE | TYPE | LENGTH | WEIGHT 45' UNIT **₩**B12 #5 | STR | 22'-1" \* S4 100 100 747 2 7'-2" \* S5 #5 5′-8″ 47 \* EPOXY COATED REINFORCING STEEL LBS. 1715 CLASS AA CONCRETE CU.YDS. 11.5 TOTAL VERTICAL CONCRETE BARRIER RAIL 90.0 LN. FT.





END VIEW

SIDE VIEW



3/2/2018

Mode M. Lepone

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### NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS. LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

CONCRETE	RELE	EASE	STRENGTH
UNIT			PSI
45' UNITS			4000

B-5817 PROJECT NO.\_ **ANSON** COUNTY 15+17.00 -L-STATION:

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

3'-0" X 1'-9" PRESTRESSED CONCRETE CORED SLAB UNIT (SPAN A)

	SHEET NO.					
BY:	BY: DATE: NO. BY: DATE:					
		3			TOTAL SHEETS	
		4			26	

STD. NO. 21" PCS3\_33\_90S

3'-9" "GUTTERLINE ' RAIL HEIGHT'

ASSEMBLED BY : MLO CHECKED BY : \_ DRAWN BY: DGE 5/09

CHECKED BY : BCH 6/09

\_ DATE : <u>1-18</u> \_\_ DATE : <u>1-18</u> DESIGN ENGINEER OF RECORD : NML DATE : 3-18

CONST. JT.

REV. II/I4

MAA/TMG

VARIES ( THICKNES

CHAMFER └─#5 S3 (SEE ``PLAN OF UNIT" FOR SPACING)

GROUT—

SECTION T-T

AT OPEN JOINT AT BENT

(THIS IS TO BE USED WHERE

FOAM JOINT IS NOT USED)

© OPEN JT. IN— RAIL @ BENT

ELEVATION AT EXPANSION JOINTS

© 1/2"EXP. JT. MAT'L HELD IN PLACE WITH GALVANIZED NAILS.

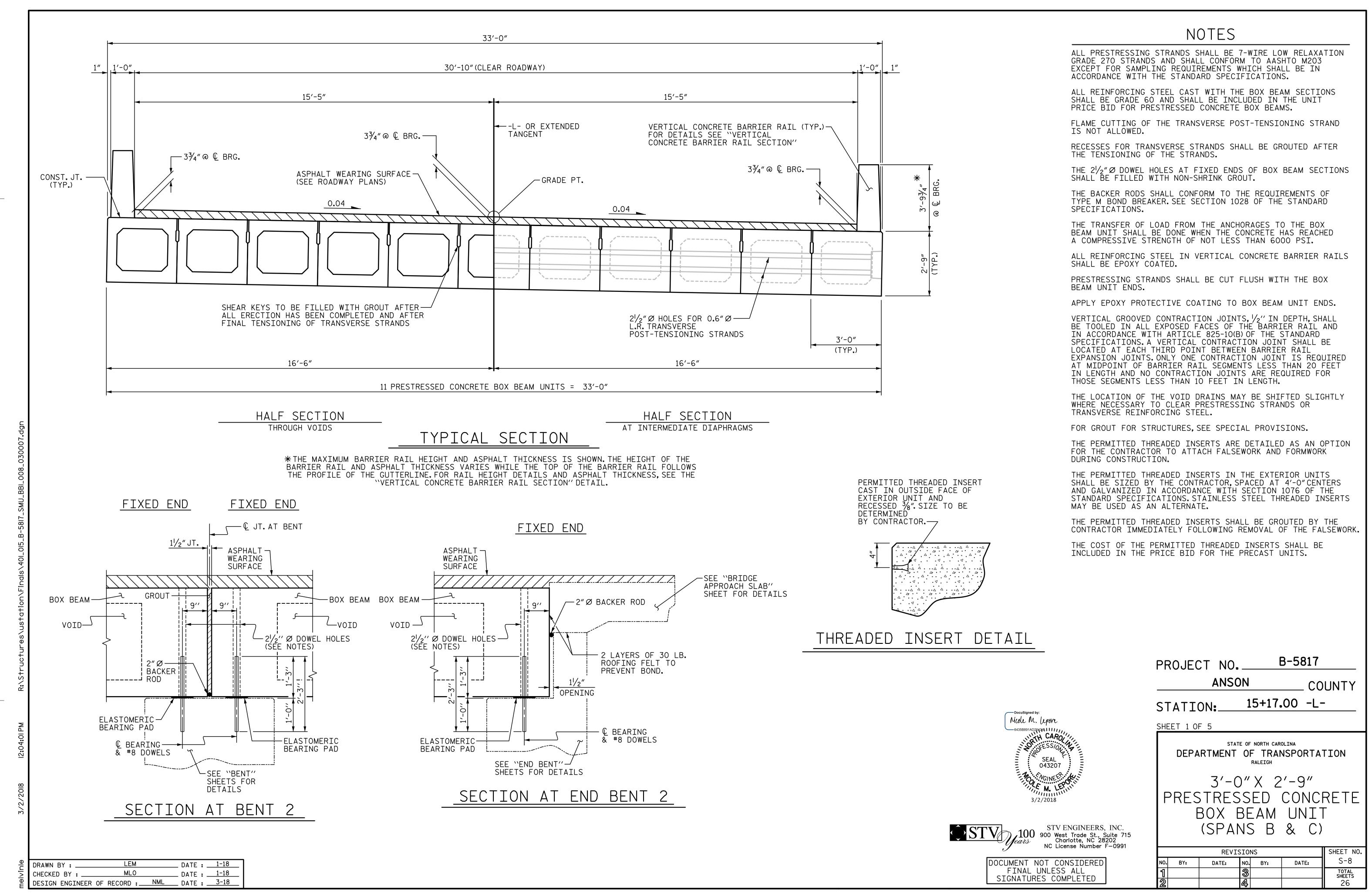
(NOTE: OMIT EXP. JT. MAT'L.

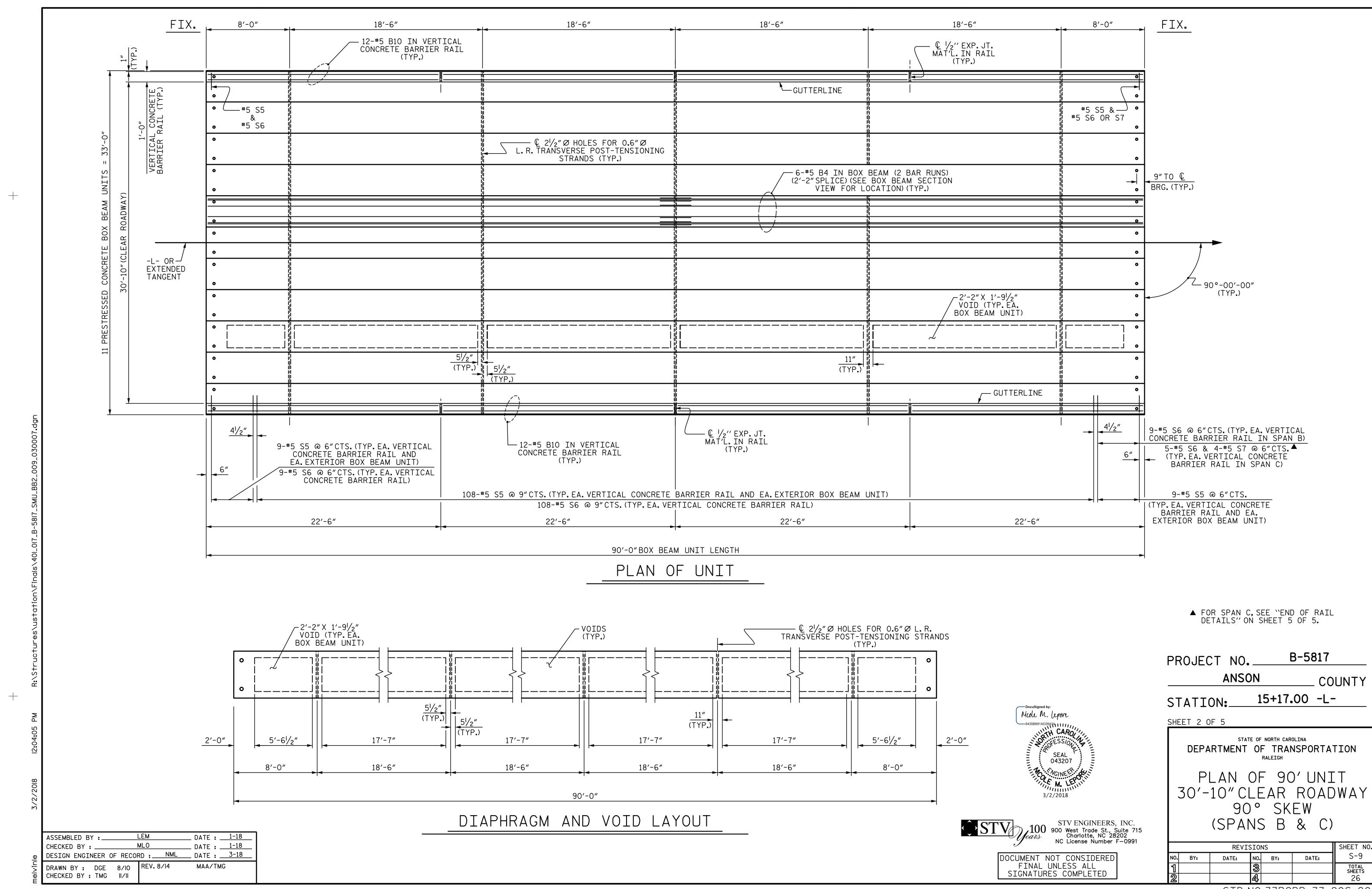
WHEN SLIP FORM IS USED)

CHAMFER

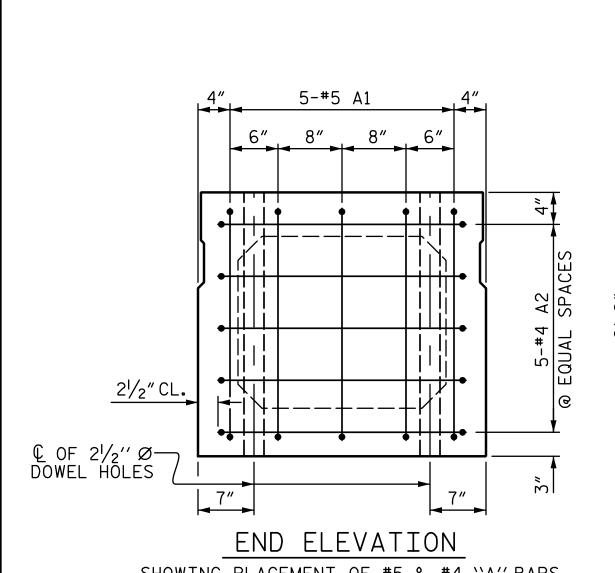
CONCRETE BARRIER RAIL SECTION

END OF RAIL DETAILS

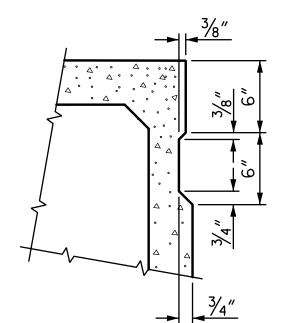




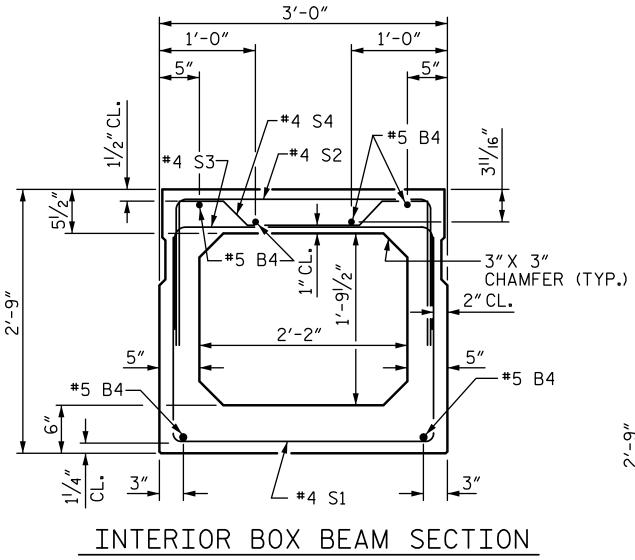
STD.NO.33PCBB\_33\_90S\_90L



SHOWING PLACEMENT OF #5 & #4 '`A'' BARS AND LOCATION OF DOWEL HOLES. (INTERIOR BOX BEAM SECTION SHOWN-EXTERIOR SECTION SIMILAR EXCEPT SHEAR KEY LOCATION. STRAND LAYOUT NOT SHOWN.)



SHEAR KEY DETAIL NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR BOX BEAMS.



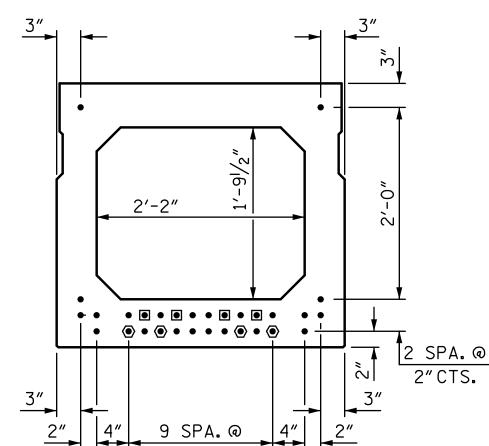
(STRAND LAYOUT NOT SHOWN)

## 3'-0" CL. #5 S5 — #4 S2¬ #4 S47 #4 S37 <del>√</del> #5 B4 -CHAMFER (TYP.) 2"CL. ∠ #4 S1

EXTERIOR BOX BEAM SECTION (STRAND LAYOUT NOT SHOWN)

GRADE 270 STRANDS 0.6"Ø L.R. 0.217 (SQUARE INCHES) ULTIMATE STRENGTH (LBS.PER STRAND) 58,600 APPLIED PRESTRESS 43,950 (LBS.PER STRAND)

# O.6" Ø LOW RELAXATION STRAND LAYOUT

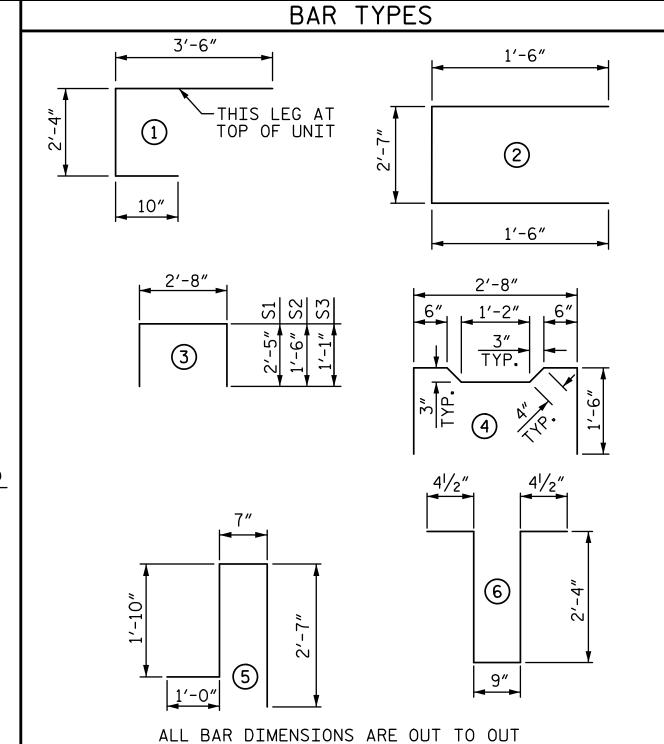


TYPICAL STRAND LOCATION (30 STRANDS REQUIRED)

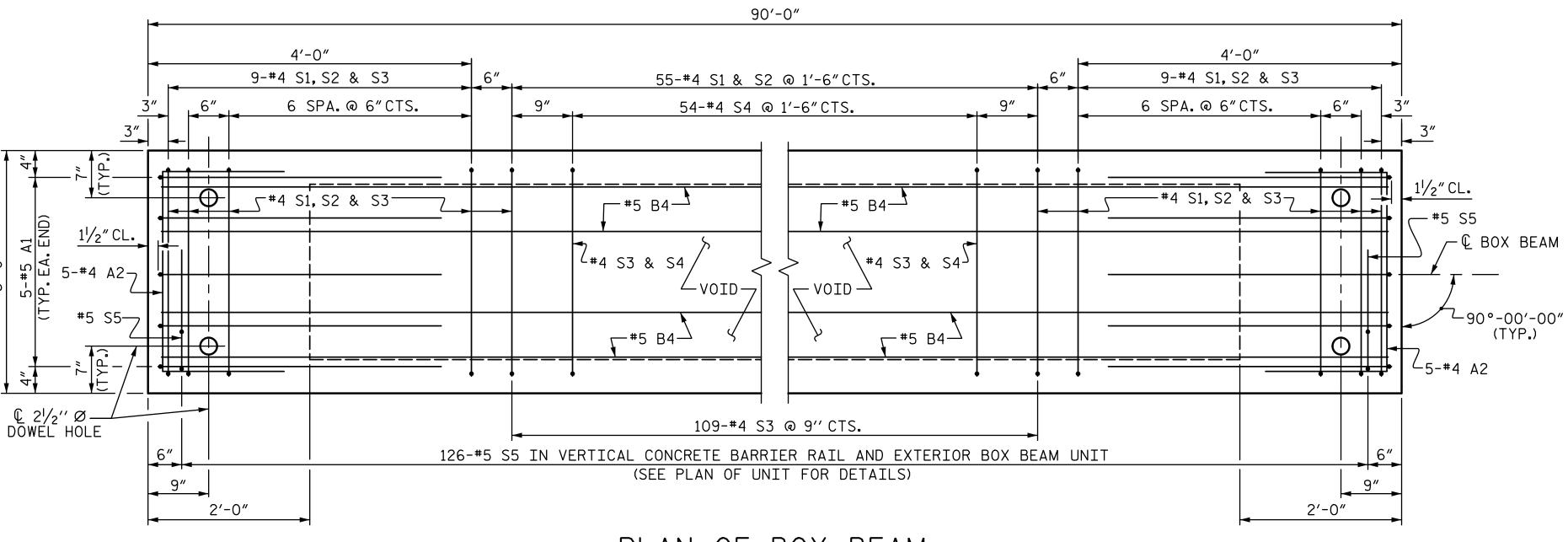
### DEBONDING LEGEND

- FULLY BONDED STRANDS
- STRANDS DEBONDED FOR 4'-0"FROM END OF GIRDER
- STRANDS DEBONDED FOR 12'-0"FROM END OF GIRDER

BOND SHALL BE BROKEN ON STRANDS AS SHOWN FOR THE SPECIFIED LENGTH FROM EACH END OF THE BOX BEAM. SEE STANDARD SPECIFICATIONS ARTICLE 1078-7.

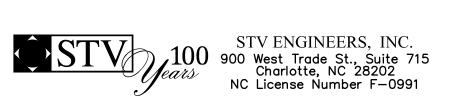


BI	_L OF	MATER	RIAL F	FOR ONE	BOX BE	EAM SEC	TION
				EXTERI	OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
A1	10	#5	1	6′-8″	70	6′-8″	70
A2	40	#4	2	5′-7″	149	5′-7″	149
B4	12	#5	STR	45′-11″	575	45′-11″	575
K1	15	#4	6	6′-2″	62	6′-2″	62
K2	10	#4	STR	2′-7″	17	2′-7″	17
S1	73	#4	3	7′-6″	366	7′-6″	366
S2	73	#4	3	5′-8″	276	5′-8″	276
S3	127	#4	3	4'-10"	410	4'-10"	410
S4	54	#4	4	5′-10″	210	5′-10″	210
* S5	126	#5	5	6′-0″	789		
	ORCING :			2135	LBS.	21	35 LBS
	<u>xy coate</u>		F. STEEL		LBS.		
8000	P.S.I. CO	<u>NCRETE</u>		16.0	CU. YDS.	15.9	CU. YDS
0.6"Ø	L.R. STR	ANDS		No. 30		No. 30	



PLAN OF BOX BEAM

EXTERIOR UNIT SHOWN, INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S5 BARS. FOR LOCATION OF DIAPHRAGMS, SEE "PLAN OF UNIT". FOR THREADED INSERTS, SEE "THREADED INSERT DETAIL".
FOR REINFORCING STEEL IN DIAPHRAGMS, SEE "DOUBLE DIAPHRAGM DETAILS".



3/2/2018

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Mode M. lepon

B-5817 PROJECT NO. \_\_ ANSON COUNTY 15+17.00 -L-STATION:

SHEET 3 OF 5

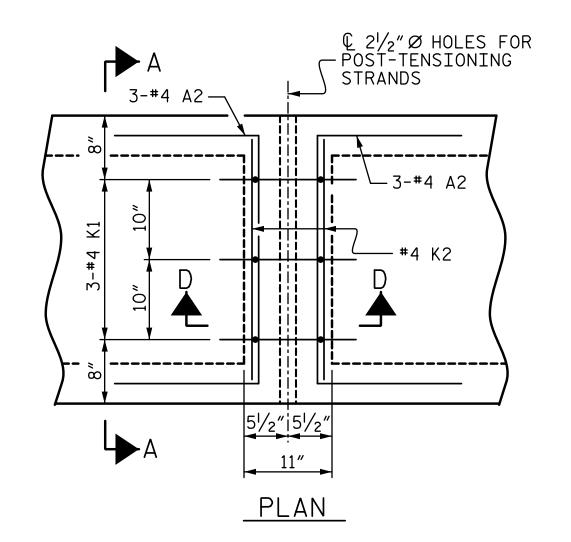
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

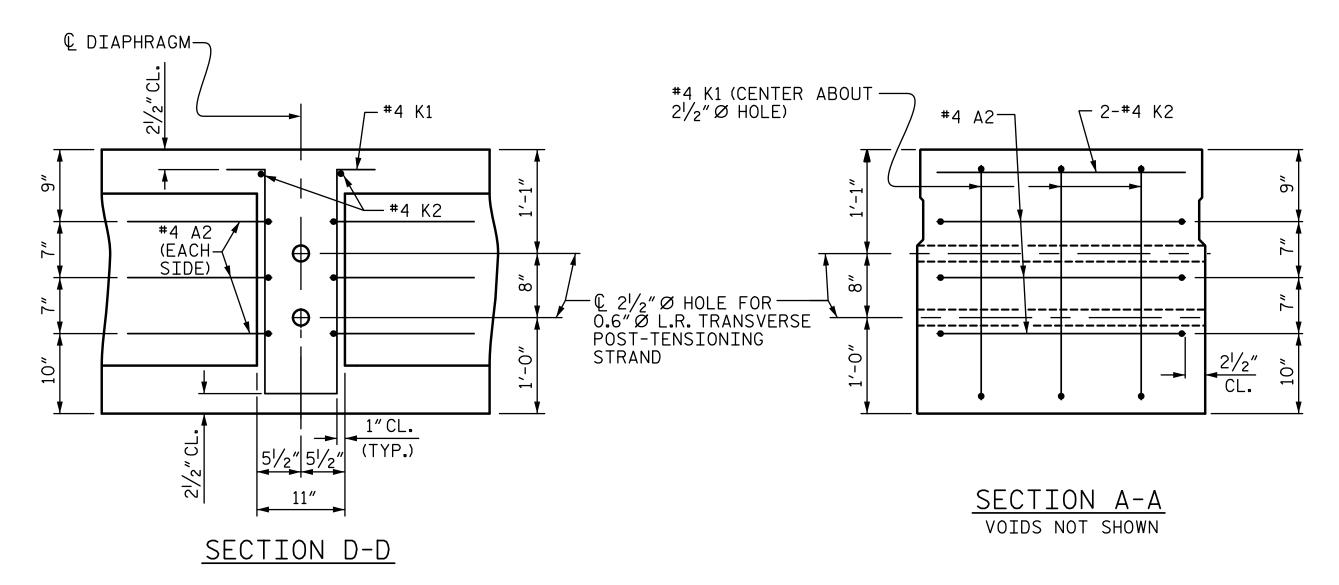
3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT (SPANS B & C)

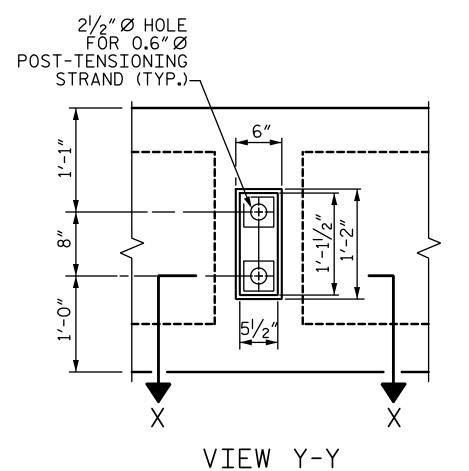
REVISIONS					SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-10
		3			TOTAL SHEETS
		4			26

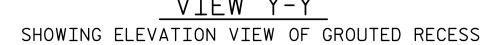
STD. NO. 33PCBB4\_90S\_90L

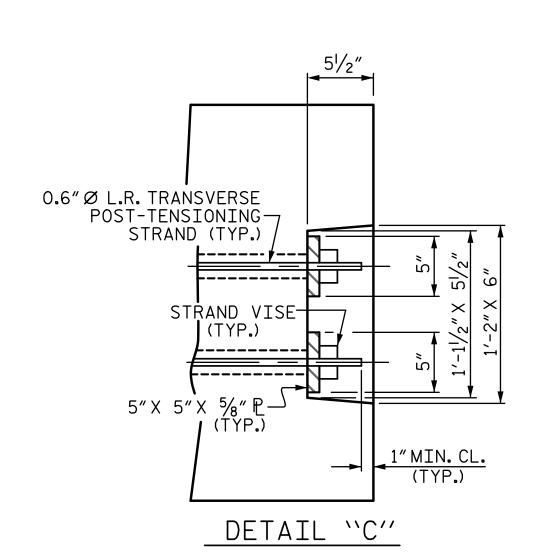
LEM \_ DATE : <u>1-18</u> ASSEMBLED BY : \_ DATE : <u>1-18</u> MLO CHECKED BY : \_ DESIGN ENGINEER OF RECORD : NML DATE : 3-18 REV. 9/I4 MAA/TMG DRAWN BY : DGE 10/11 CHECKED BY : TMG II/II

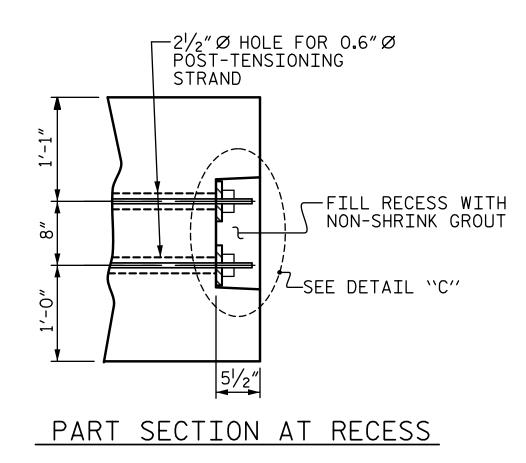


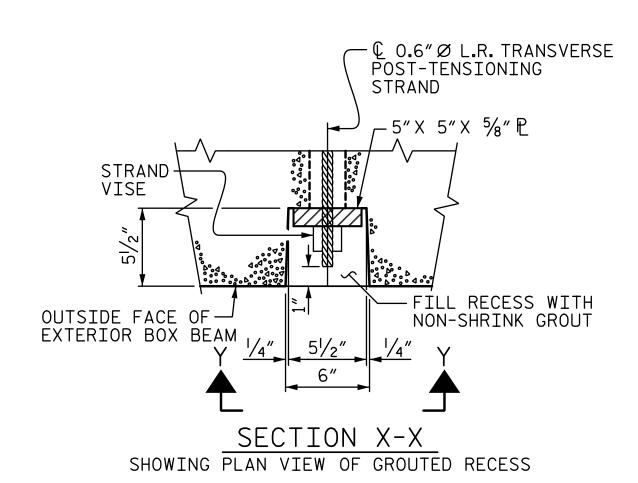












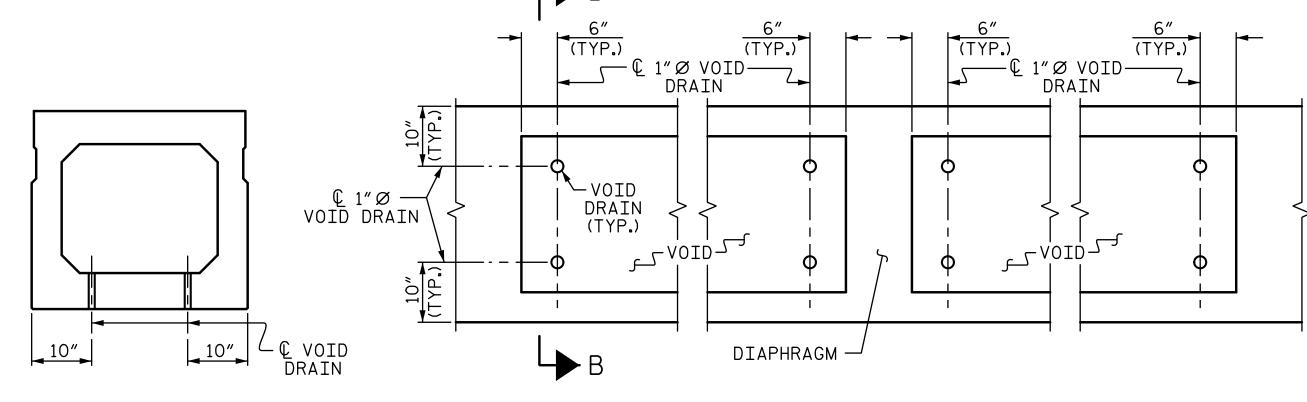
## GROUTED RECESS DETAIL AT END OF POST-TENSIONED STRANDS OF EXTERIOR BOX BEAM

## DOUBLE DIAPHRAGM DETAILS

#4 "S" BARS NOT SHOWN. #4 "S" BARS MAY BE SHIFTED SLIGHTLY TO CLEAR  $2^{1}/_{2}$ " Ø HOLE.

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-9"
90'BOX BEAM UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	2¾″ ∤
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD***	3⁄4″ ♦
FINAL CAMBER	2″ 🛉

\*\* INCLUDES FUTURE WEARING SURFACE



SECTION B-B

PART PLAN

## VOID DRAIN DETAILS

(DIMENSIONS SHOWN ARE TYPICAL FOR EACH VOID)



Mode M. Lepone

PROJEC	T NO	B-58	817
	ANSON		COUNT
STATIO	)N <b>:</b>	15+17.00	-L-

SHEET 4 OF 5

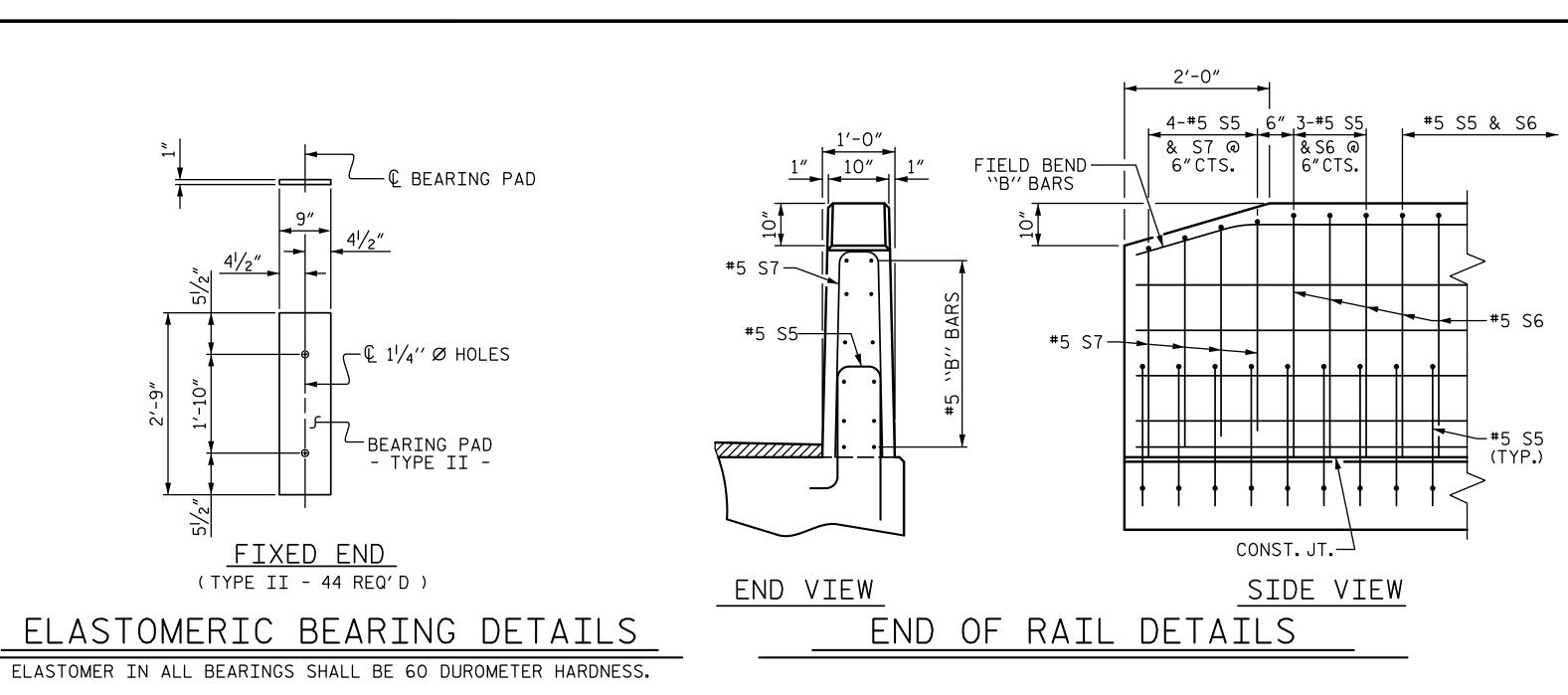
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

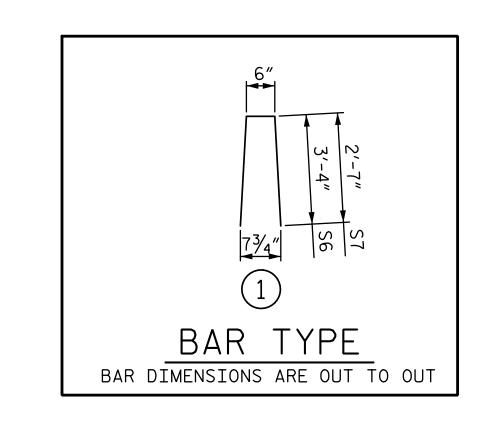
3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT (SPANS B & C)

	REVIS	SHEET NO.			
<b>/:</b>	DATE:	NO.	BY:	DATE:	S-11
		<b>®</b>			TOTAL SHEETS
		<b>જ</b>			26

\_\_ DATE : <u>1-18</u> ASSEMBLED BY : \_\_ DATE : <u>1-18</u> MLO CHECKED BY : \_\_ DESIGN ENGINEER OF RECORD : NML DATE : 3-18 MAA/TMG DRAWN BY: DGE 10/11 REV. 8/14 CHECKED BY : TMG II/II

STD.NO.33PCBB5\_90S

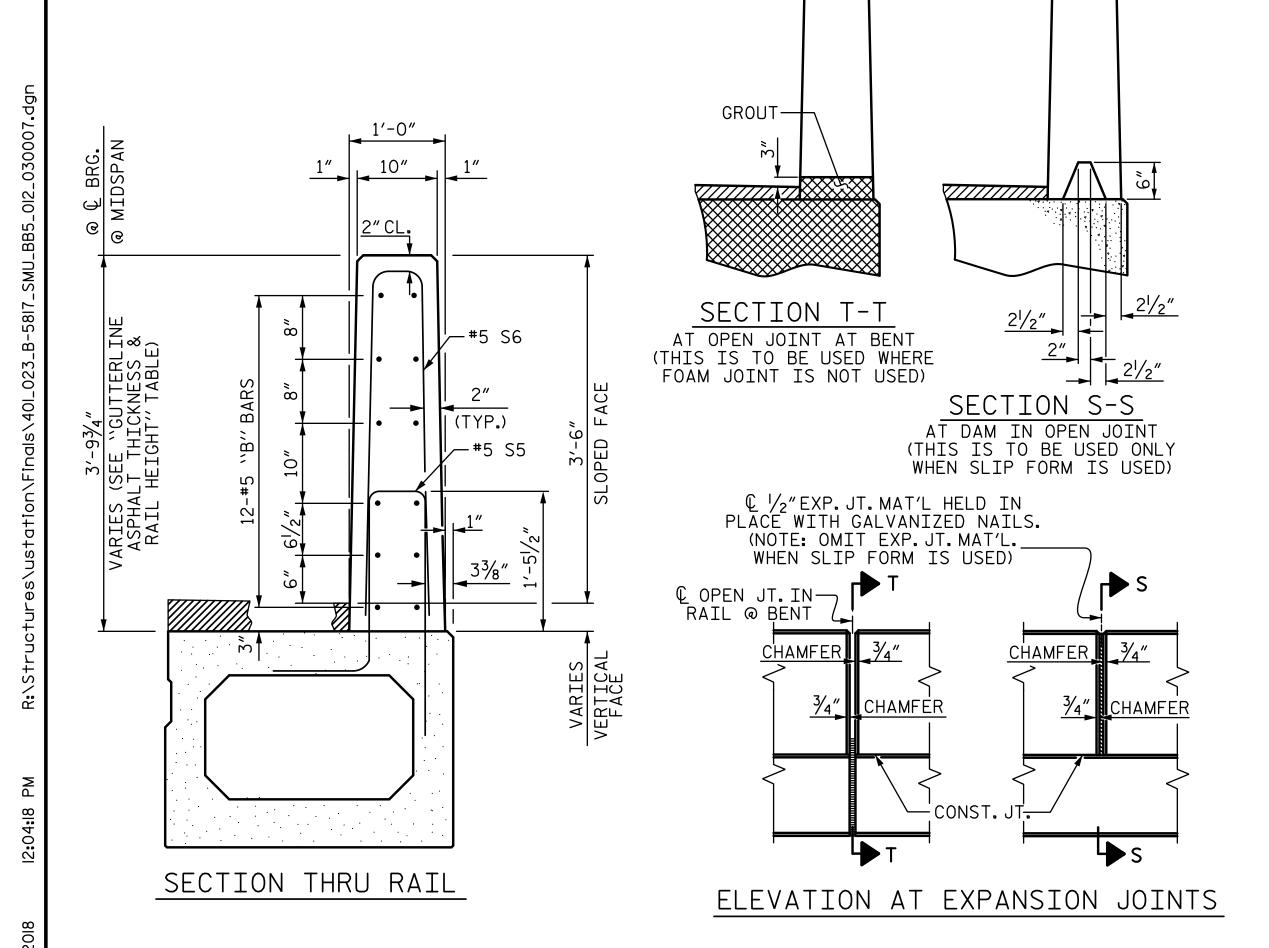




BOX BEAM UNITS REQUIRED					
	NUMBER	LENGTH	TOTAL LENGTH		
EXTERIOR B.B.	4	90'-0"	360′-0″		
INTERIOR B.B.	18	90′-0″	1620'-0"		
TOTAL	22		1980′-0″		

BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL								
BAR	BARS PER PAIR OF	EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT	
	SPAN B	SPAN C						
<b></b> ₩B10	96	96	192	#5	STR	22'-1"	4422	
<b>*</b> S6	252	244	496	#5	1	7′-2″	3708	
* S7	-	8	8	#5	1	5′-8″	47	
* EPOXY COATED REINFORCING STEEL LBS.						8177		
CLASS AA CONCRETE						1	46.6	
TOTAL VERTICAL CONCRETE BARRIER RAIL LN.FT.							360.0	

GUTTERLINE A	ASPHALT	THICKNESS	& F	RAIL	HEIGHT
	AS	SPHALT OVERLAY THICKNI @ MID-SPAN	ESS		IL HEIGHT MID-SPAN
90'UNITS		13/4"			3'-73/4"



VERTICAL CONCRETE BARRIER RAIL DETAILS

STV ENGINEERS, INC.

900 West Trade St., Suite 715
Charlotte, NC 28202
NC License Number F-0991

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SIGNATURES COMPLETED

Mole M. lepore

PROJECT NO. B-5817

ANSON COUNTY

STATION: 15+17.00 -L-

SHEET 5 OF 5

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

3'-0"X 2'-9"
PRESTRESSED CONCRETE
BOX BEAM UNIT
(SPANS B & C)

REVISIONS					SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-12
		<b>®</b>			TOTAL SHEETS
		<b>જ</b>			26

STD. NO. 33PCBB8\_90S

ASSEMBLED BY: LEM DATE: 1-18
CHECKED BY: MLO DATE: 1-18
DESIGN ENGINEER OF RECORD: NML DATE: 3-18

DRAWN BY: DGE IO/II
CHECKED BY: TMG II/II

REV. 4/I5

MAA/TMG

ASSEMBLED BY :

CHECKED BY : \_\_\_\_

DRAWN BY: MAA 5/10 CHECKED BY: GM 5/10 \_\_ DATE : <u>1-18</u>

MAA/GM

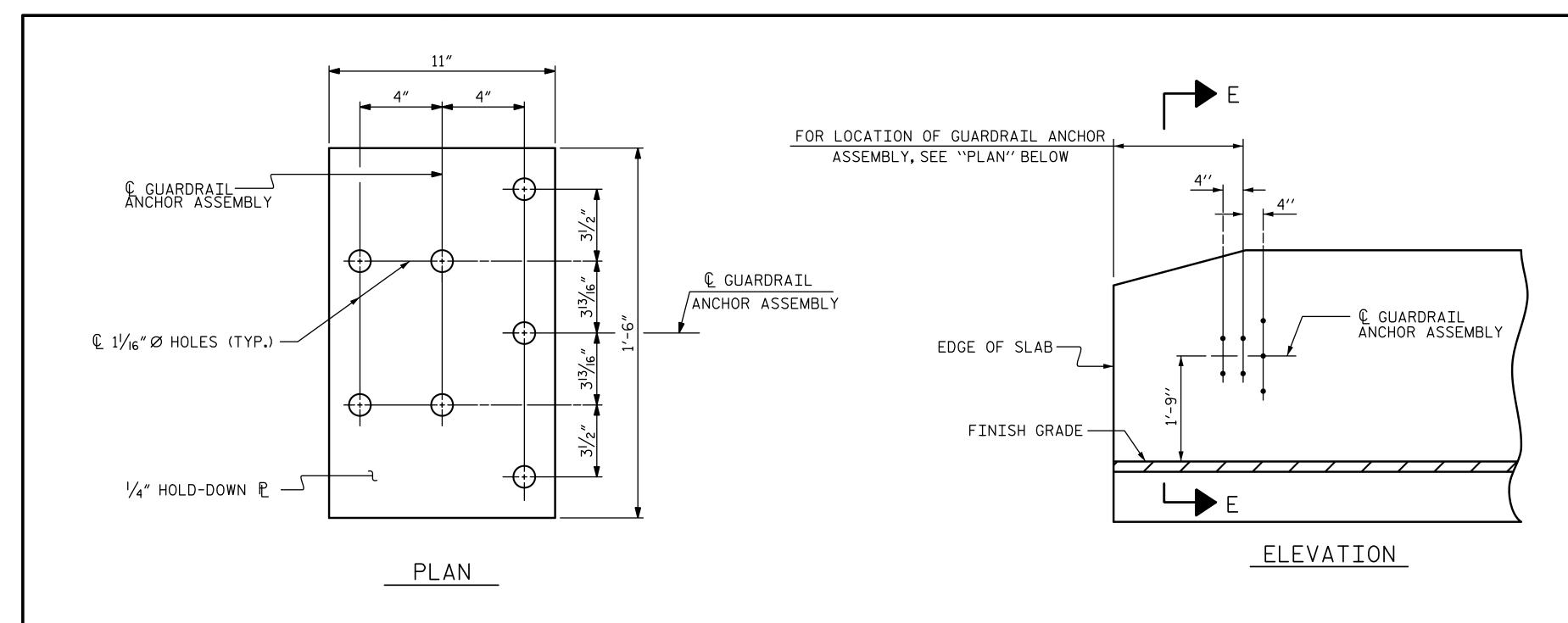
MAA/TMG

MAA/THC

MLO

REV. 6/I3 REV. I/I5 REV. I2/I7

DESIGN ENGINEER OF RECORD : NML



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 -  $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE \( \frac{7}{8} \) \( \text{\sigma} \) GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

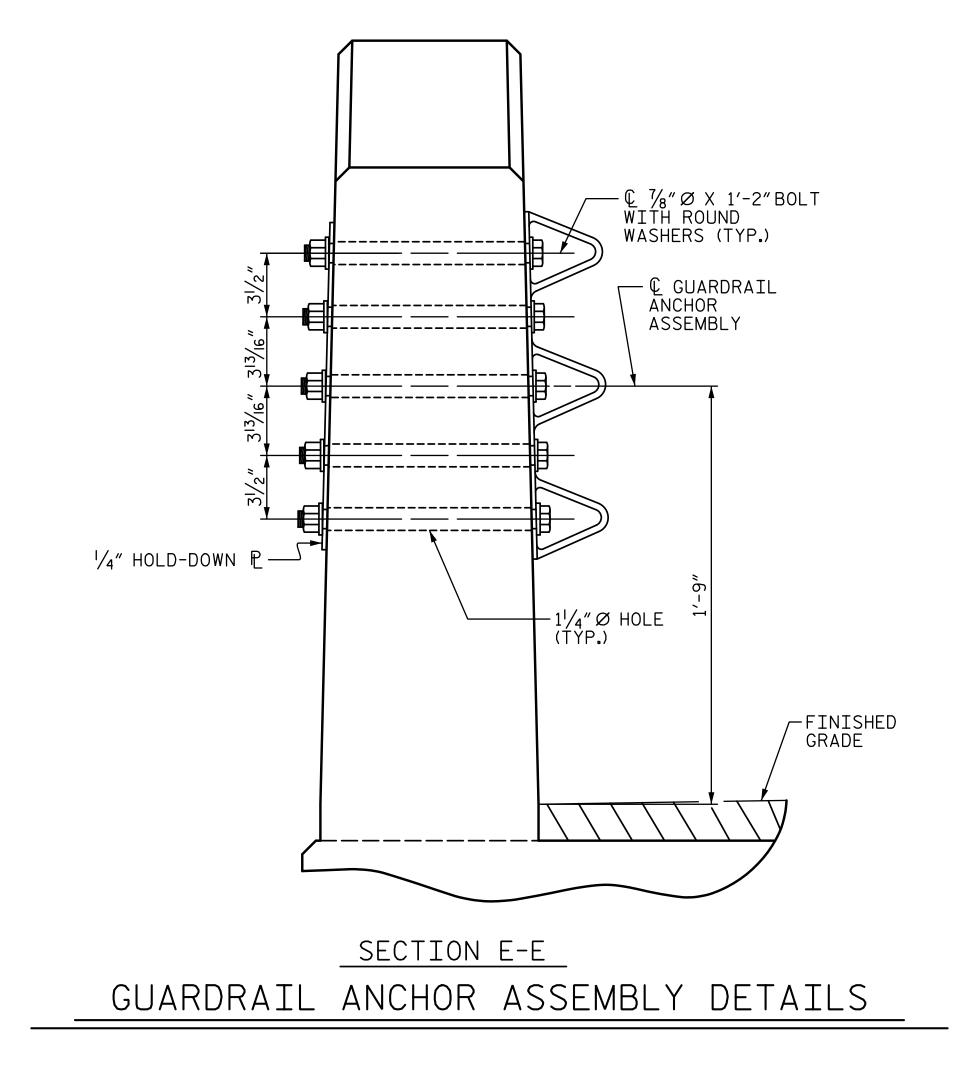
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

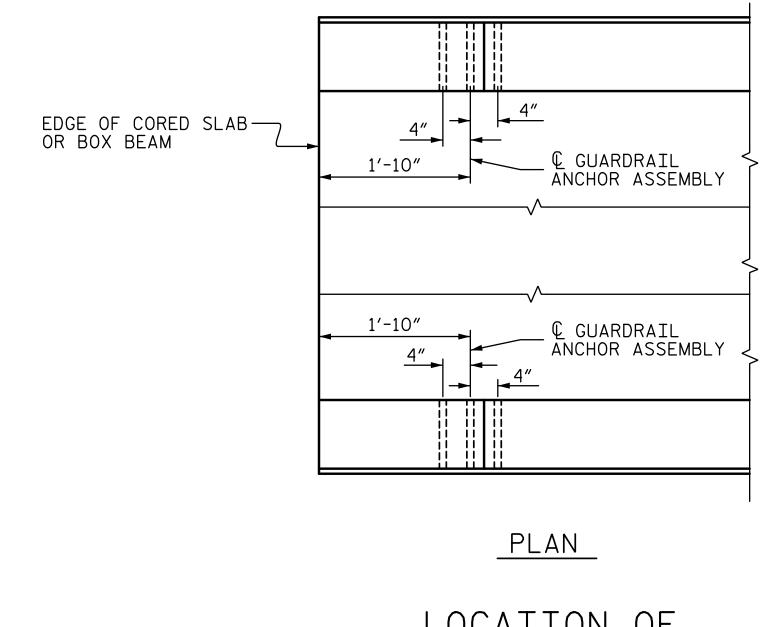
AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

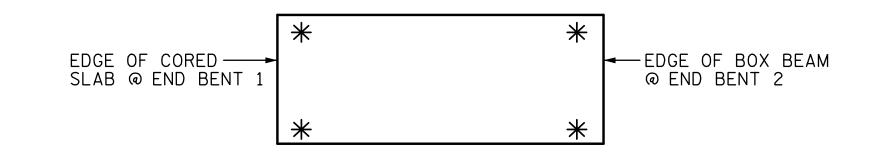
THE 1  $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.





LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



# SKETCH SHOWING POINTS OF ATTACHMENT

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

Docusigned by:

Mcole M. Lyone

6435B891AD26484.11111

CARO

SEAL

043207

STV ENGINEERS, INC.

900 West Trade St., Suite 715
Charlotte, NC 28202
NC License Number F-0991

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ANSON COUNTY

STATION: 15+17.00 -L-

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

GUARDRAIL ANCHORAGE

DETAILS

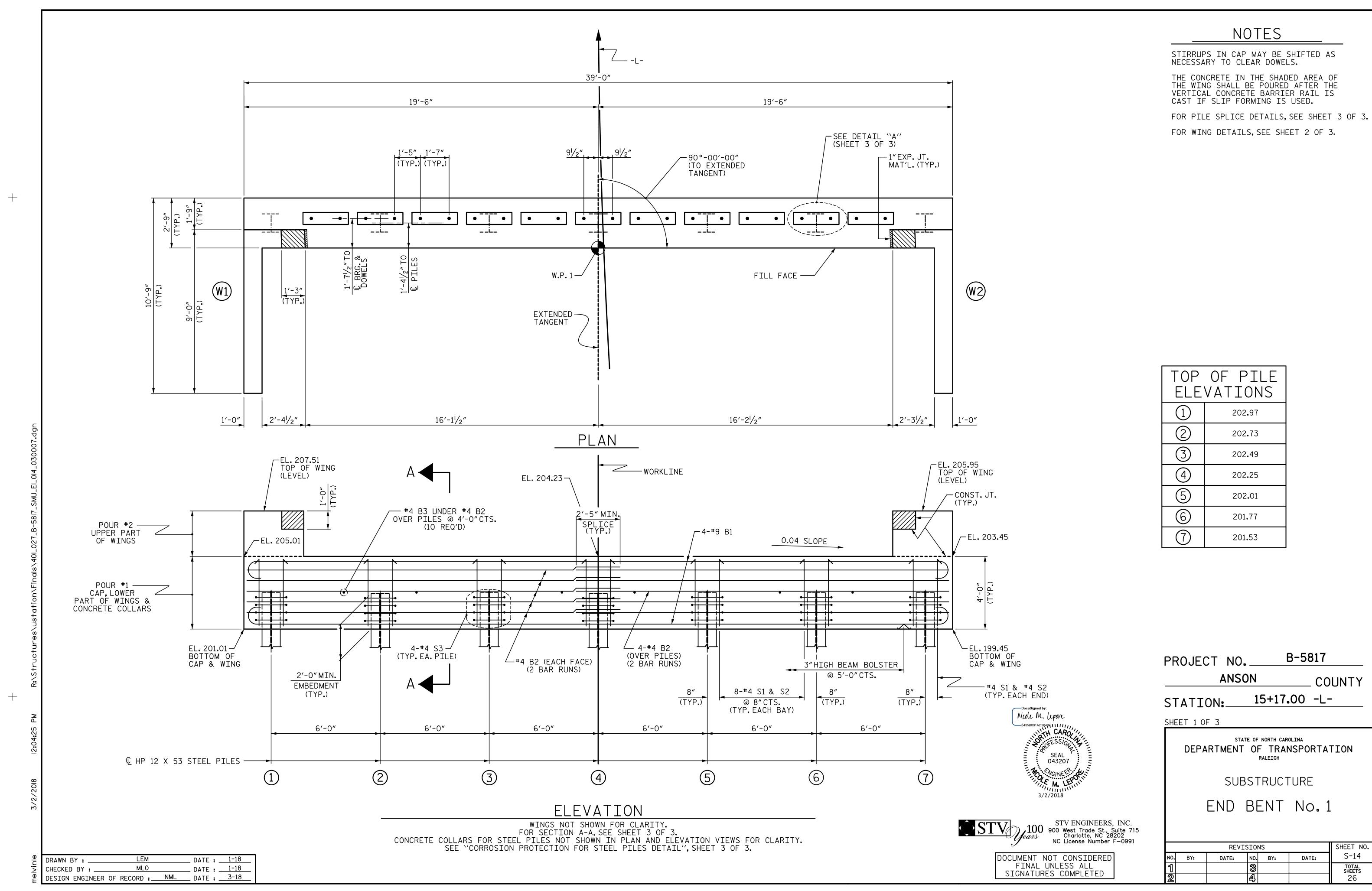
FOR VERTICAL CONCRETE

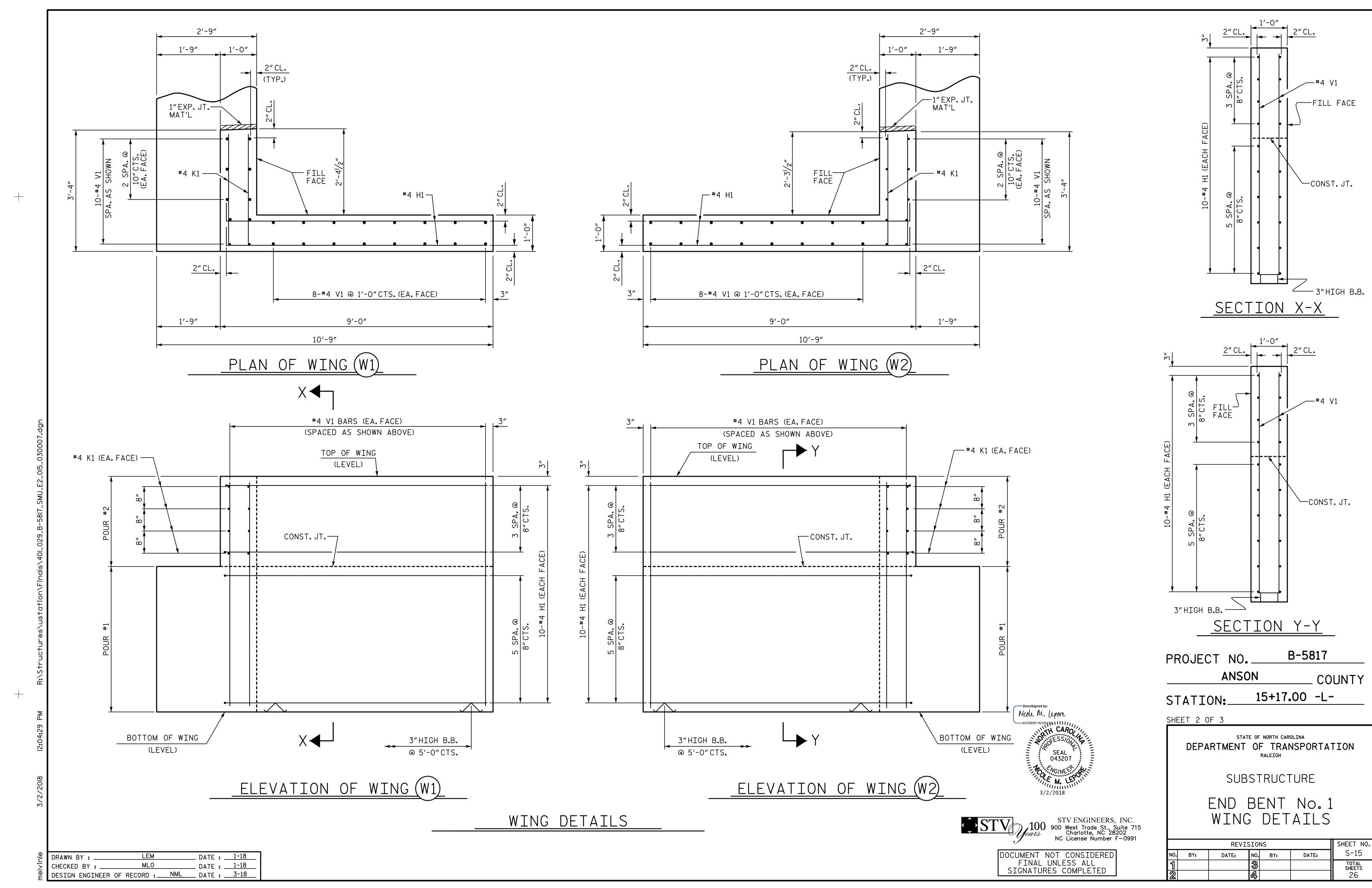
BARRIER RAIL

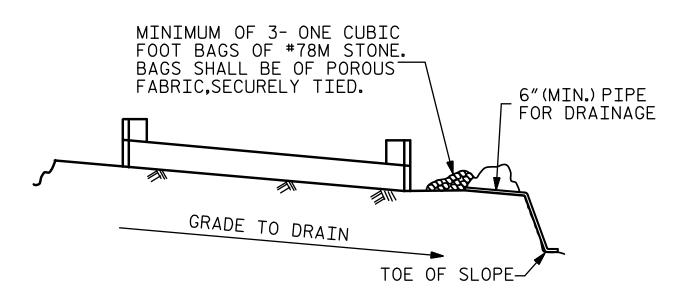
REVISIONS

NO. BY: DATE: NO. BY: DATE: S-13

1 3 TOTAL SHEETS
26





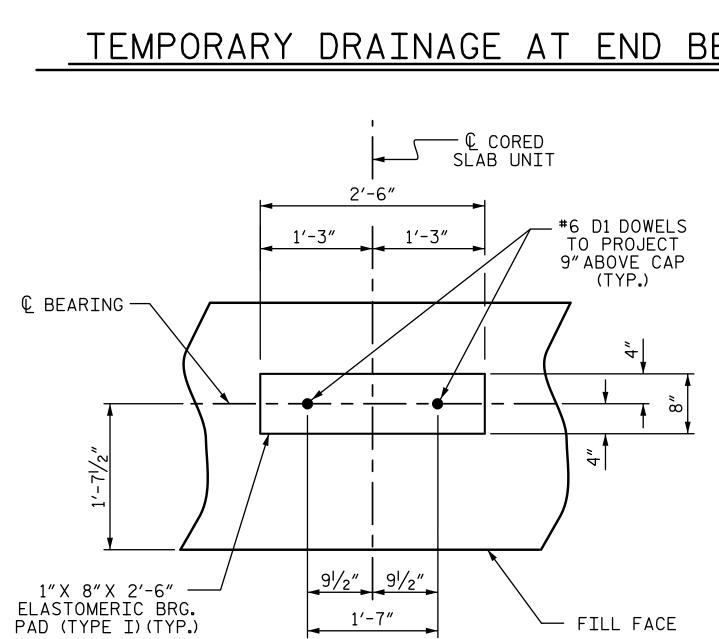


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

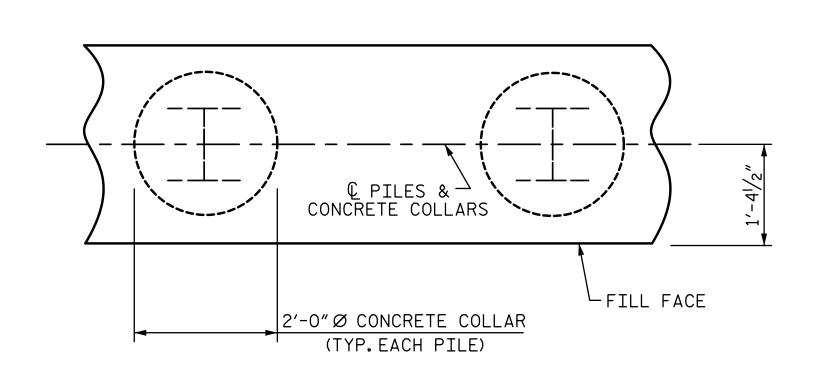
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

### TEMPORARY DRAINAGE AT END BENT



1'-7"

DETAIL "A"



PLAN ELEVATION

CORROSION PROTECTION FOR STEEL PILES DETAIL

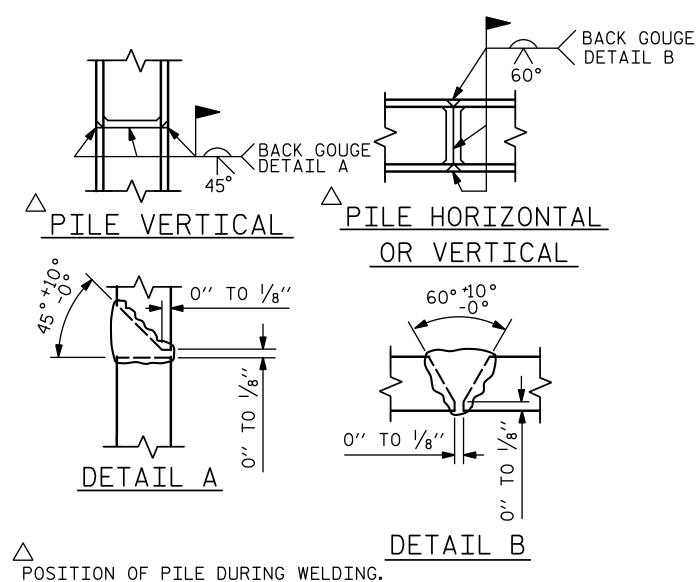
- FILL FACE

CONCRETE —

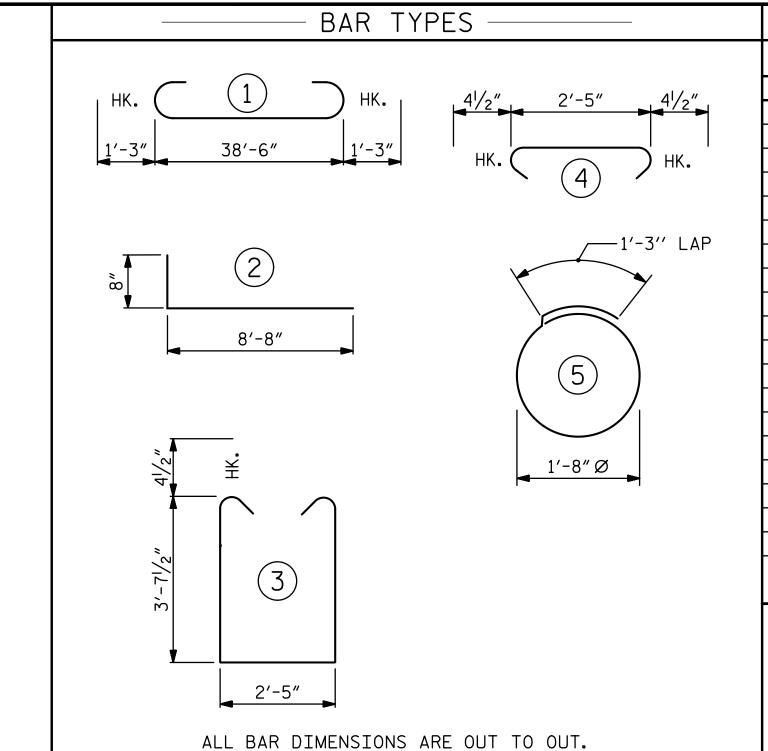
© HP 12 X 53 -STEEL PILE |

2'-0"

COLLAR



PILE SPLICE DETAILS



BILL OF MATERIAL FOR END BENT BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT #9 1 41'-0" 8 B2 28 #4 | STR | 20'-7" #4 STR 2'-5" B3 | 10 D1 | 22 | #6 | STR | 1'-6" H1 | 40 | #4 | 2 | 9'-4" K1 | 16 | #4 | STR | 2'-11" S1 | 50 #4 | 3 | 10'-5" S2 | 50 | #4 | 4 | 3'-2" S3 | 28 | #4 5 l 6′-6″ V1 | 52 | #4 | STR | 6'-2" REINFORCING STEEL CLASS A CONCRETE BREAKDOWN POUR #1 CAP, LOWER PART OF WINGS & COLLARS POUR #2 UPPER PART OF WINGS

> TOTAL CLASS A CONCRETE 21.6 C.Y. HP 12 X 53 STEEL PILES

LIN. FT.= 455.0 NO: 7 PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES

385

16

50

249

31

348

106

122

214

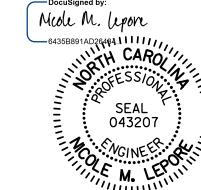
2636 LBS

19.5 C.Y.

2.1 C.Y.

-¢ #6 D1 DOWEL FILL FACE 2"CL. r#4 S2 4-#9 B1 -4-#4 B2 @ 4" CTS. 1-#4 B2 — OVER PILES EA.FACE #4 B3— #4 S1 \_\_\_\_ 2-#9 B1 2"CL.(TYP.)-2-#9 B1 € HP 12 X 53 STEEL PILE— -3"HIGH B.B. BOTTOM OF CAP  $1'-4\frac{1}{2}''$   $1'-4\frac{1}{2}''$ 2'-9"

SECTION A-A (CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")



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B-5817 PROJECT NO.\_ **ANSON** COUNTY 15+17.00 -L-STATION:

SHEET 3 OF 3

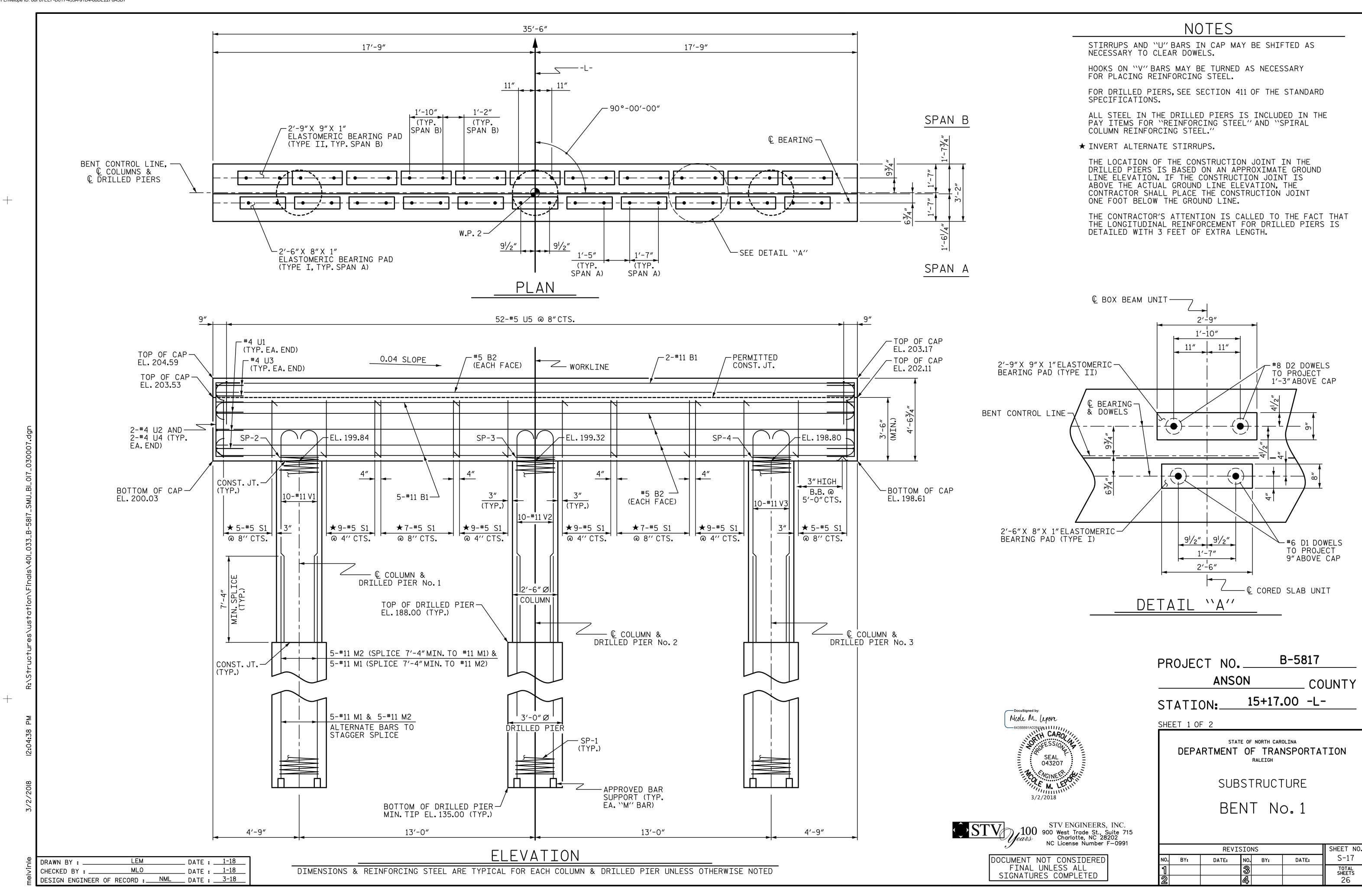
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

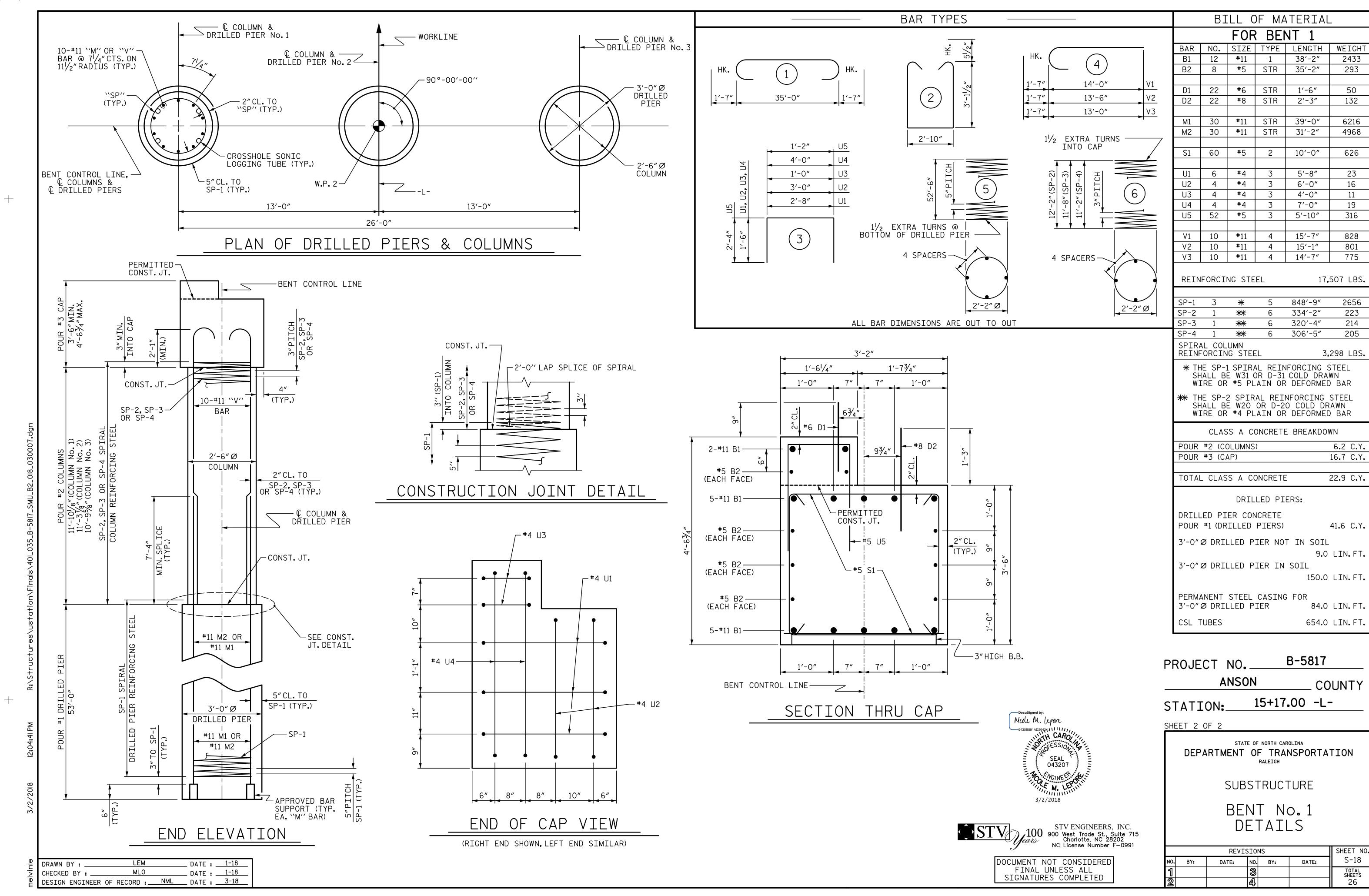
SUBSTRUCTURE

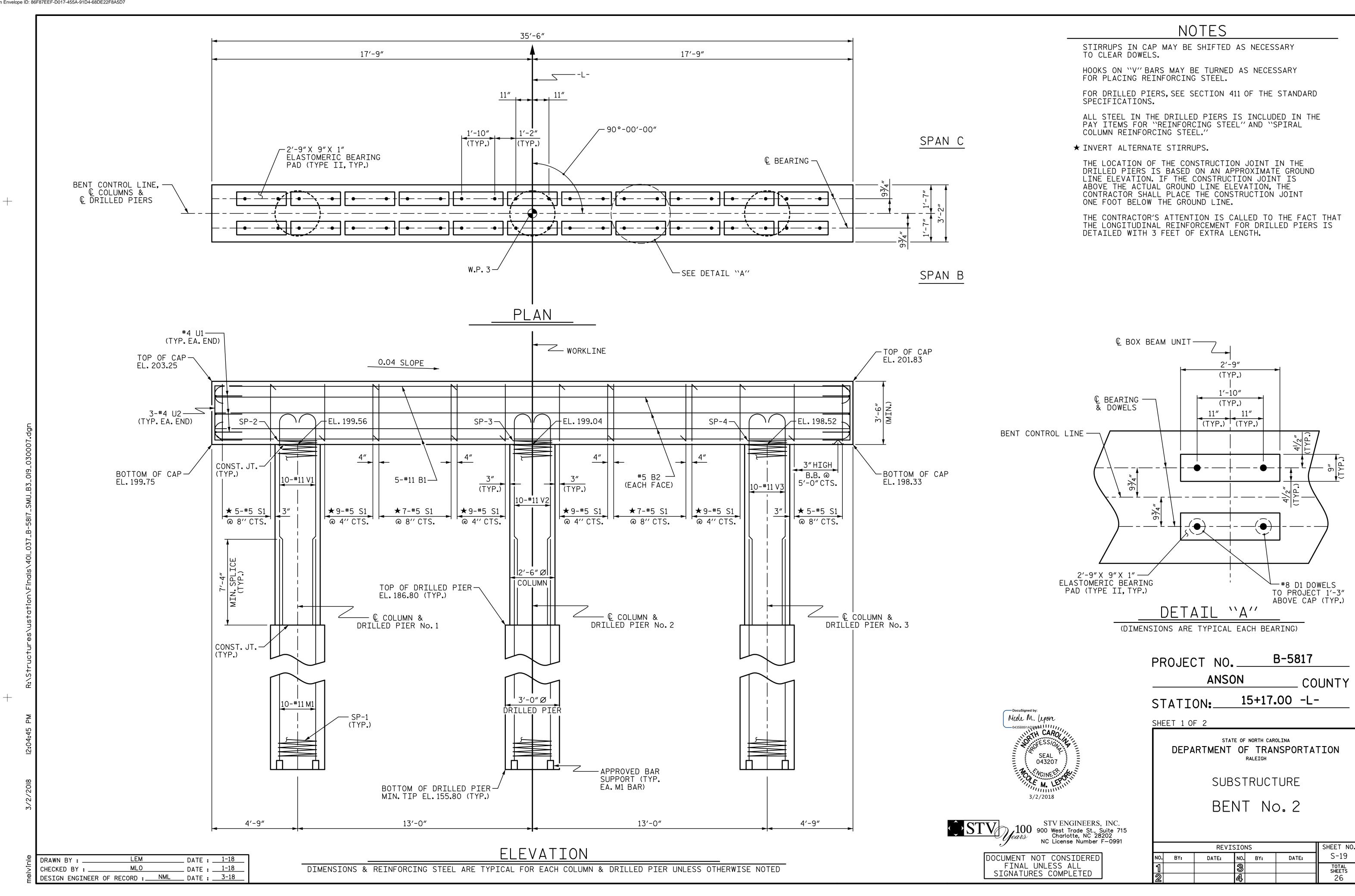
END BENT No. 1 DETAILS

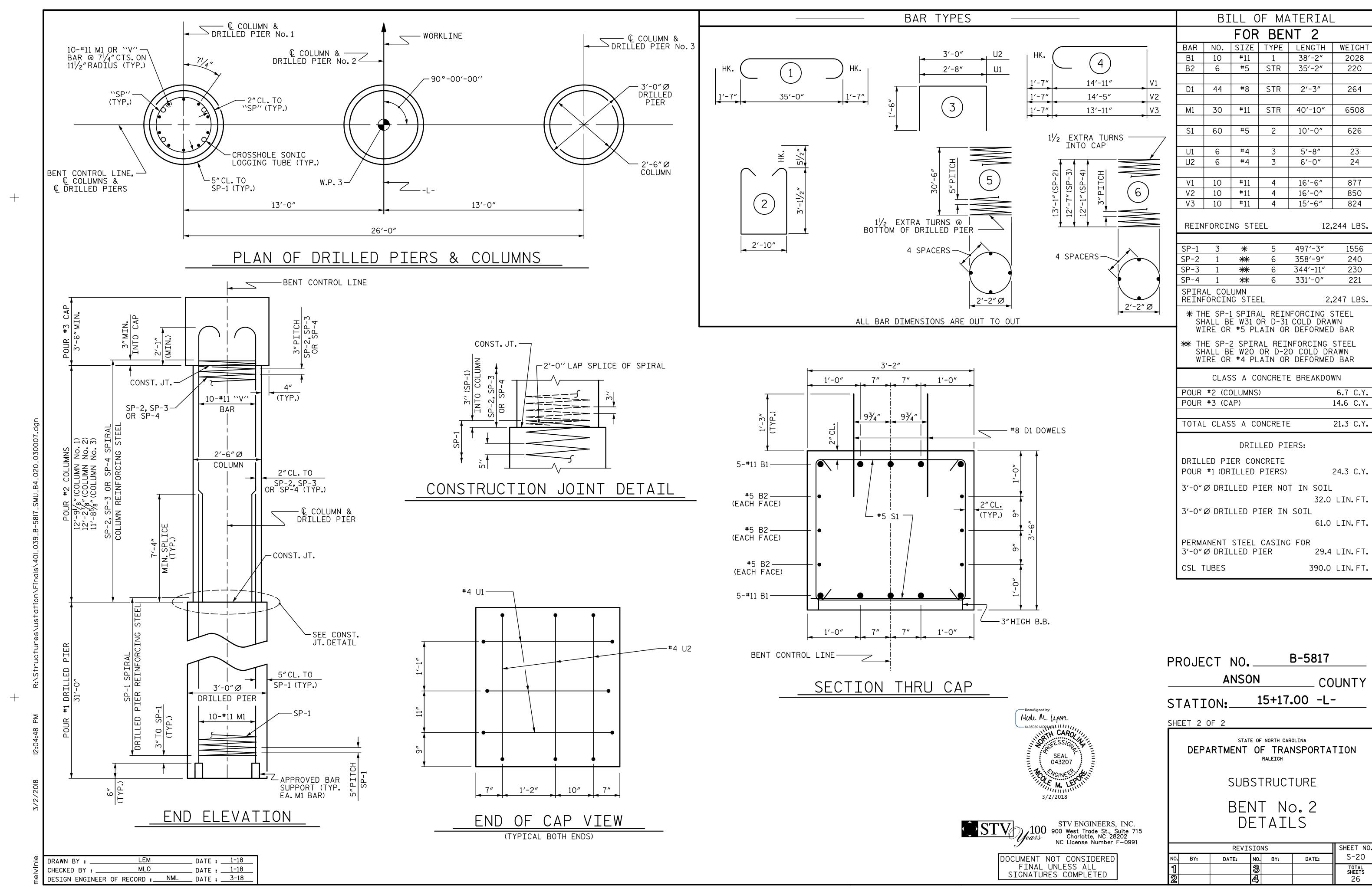
SHEET NO. REVISIONS S-16 DATE: DATE: NO. BY: NO. BY: TOTAL SHEETS 26

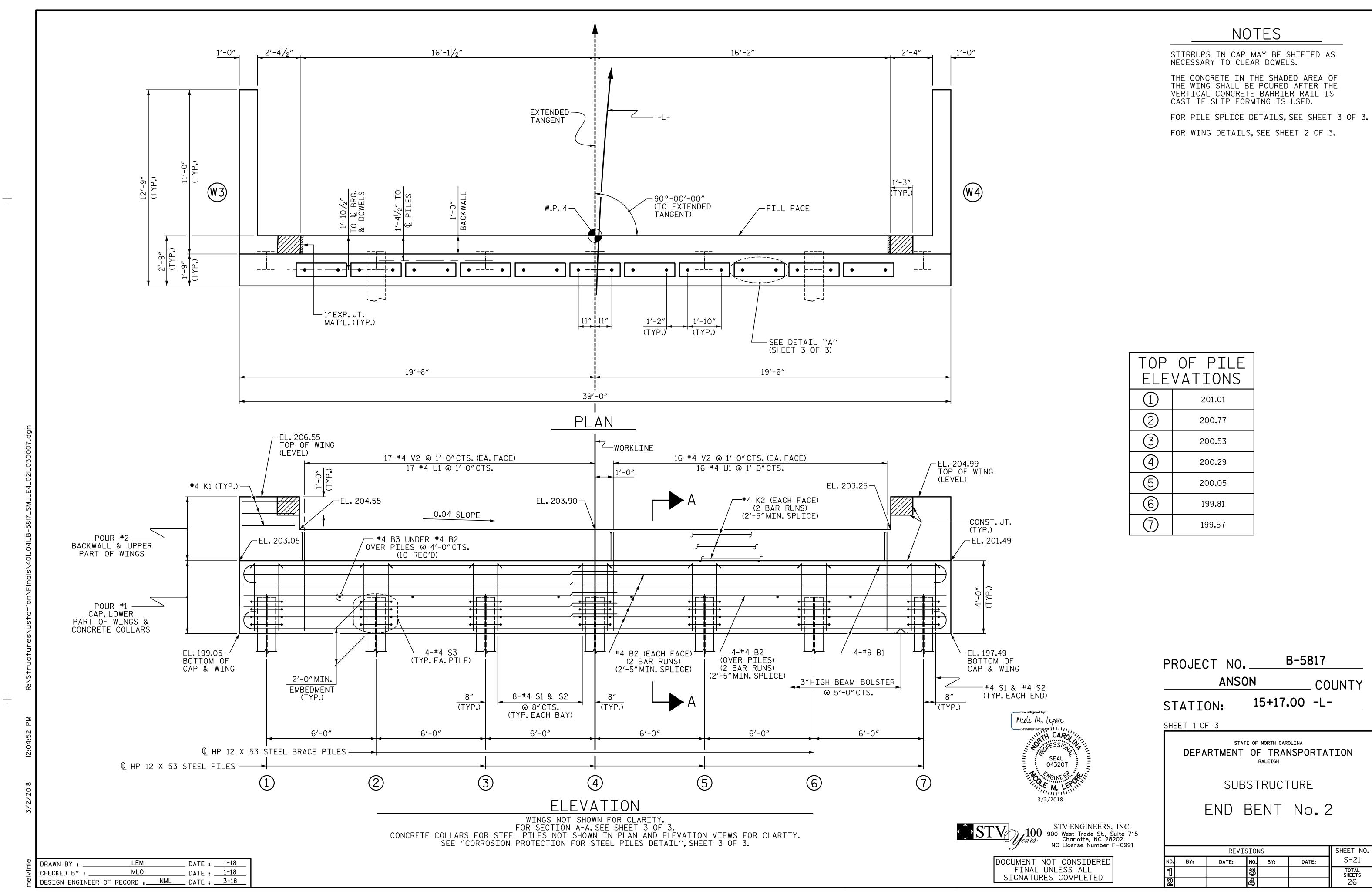
\_\_ DATE : <u>1-18</u>\_\_ DRAWN BY : \_\_\_\_ DATE : <u>1-18</u> MLO DESIGN ENGINEER OF RECORD : NML DATE : 3-18

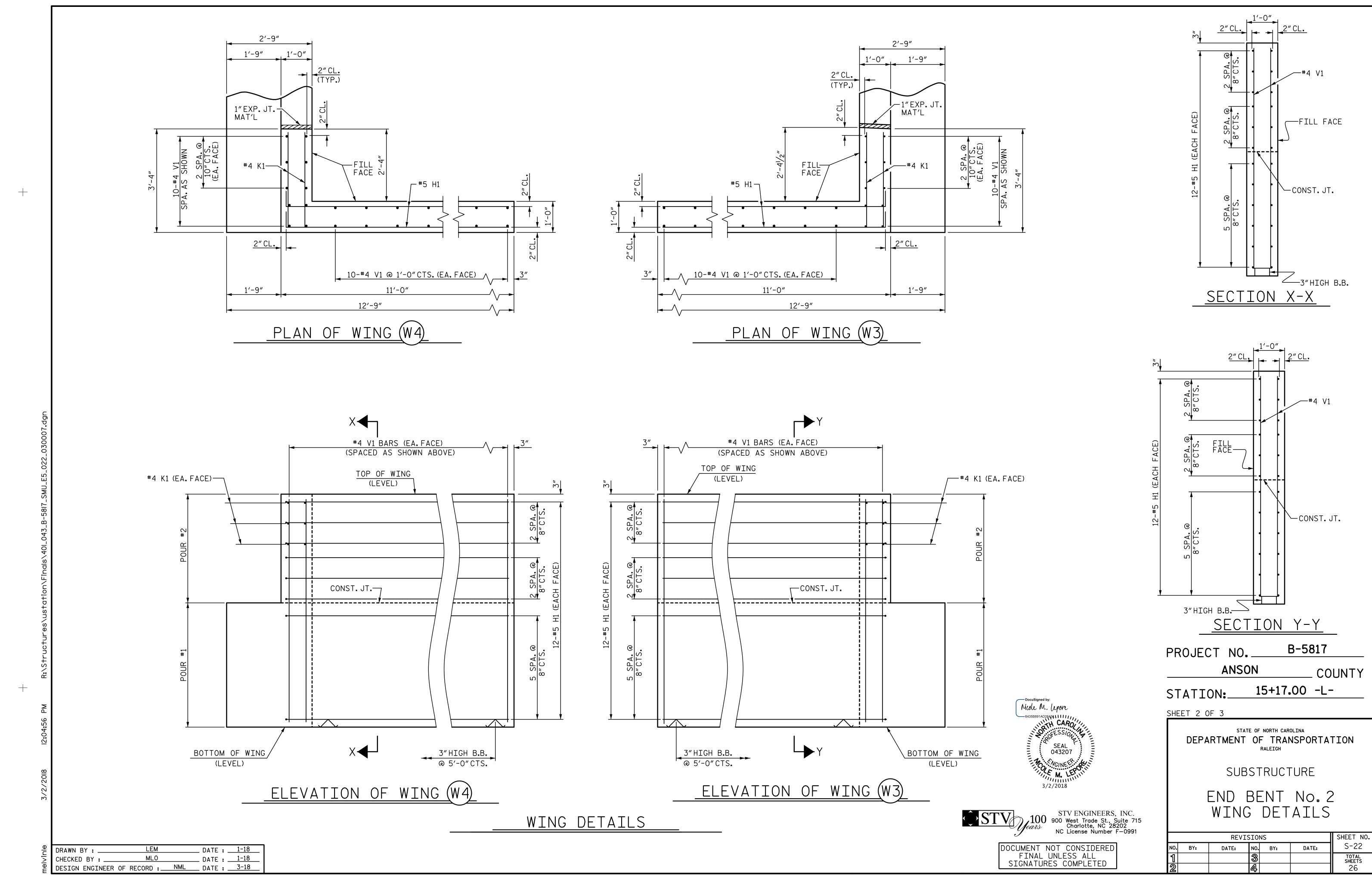


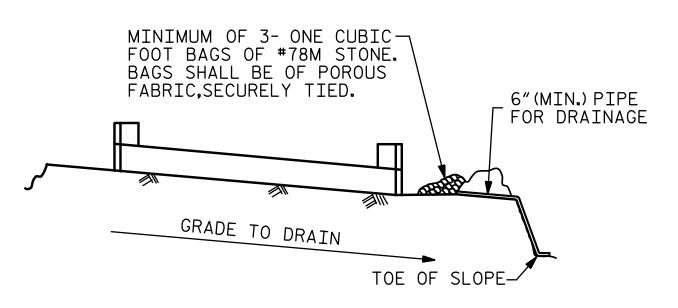










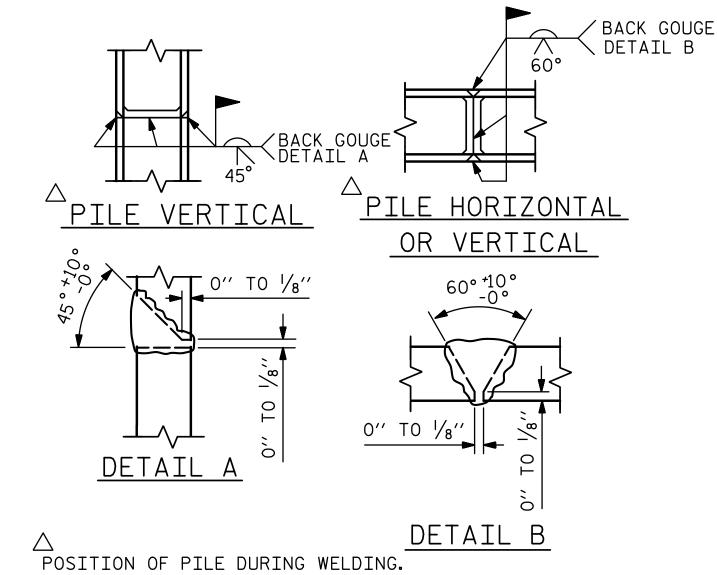


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

### TEMPORARY DRAINAGE AT END BENT



PILE SPLICE DETAILS

EA. FACE

BAR TYPES 10'-8" 1′-8″Ø 2'-5" ALL BAR DIMENSIONS ARE OUT TO OUT.

FOR END BENT 2 BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT #9 1 41'-0" 8 B2 28 #4 | STR | 20'-7" 385 B3 10 #4 | STR | 2'-5" 16 D1 | 22 | #8 | STR | 2'-3" 132 H1 | 48 | #5 | 2 | 11'-4" 567 K1 | 12 #4 | STR | 2'-11" 23 K2 | 12 | #4 | STR | 20'-7" 165 S1 | 50 | #4 | 3 | 10'-5" 348 S2 | 50 #4 3′-2″ 106 4 S3 | 28 | 122 #4 5 6′-6″ U1 | 33 | #4 | 6 | 3'-7" 79 V1 | 60 | #4 | STR | 7'-2" 287 231 V2 | 66 #4 | STR | 5'-3" REINFORCING STEEL 3576 LBS

BILL OF MATERIAL

CLASS A CONCRETE BREAKDOWN

POUR #1 CAP, LOWER PART 20.1 C.Y. OF WINGS & COLLARS

POUR #2 BACKWALL & UPPER 5.4 C.Y. PART OF WINGS

TOTAL CLASS A CONCRETE 25.6 C.Y.

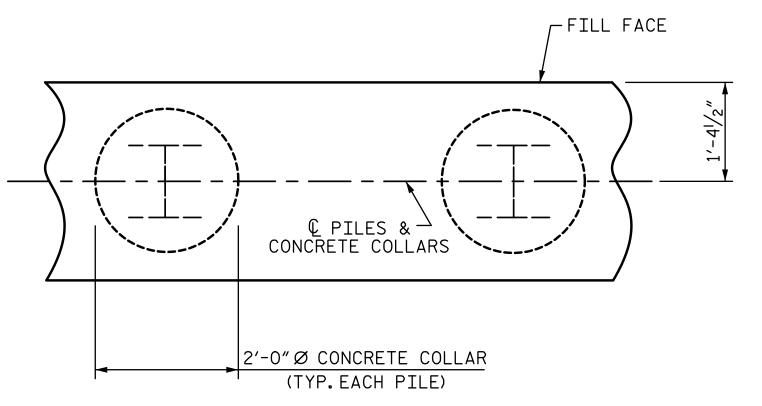
HP 12 X 53 STEEL PILES

LIN. FT.= 175.0

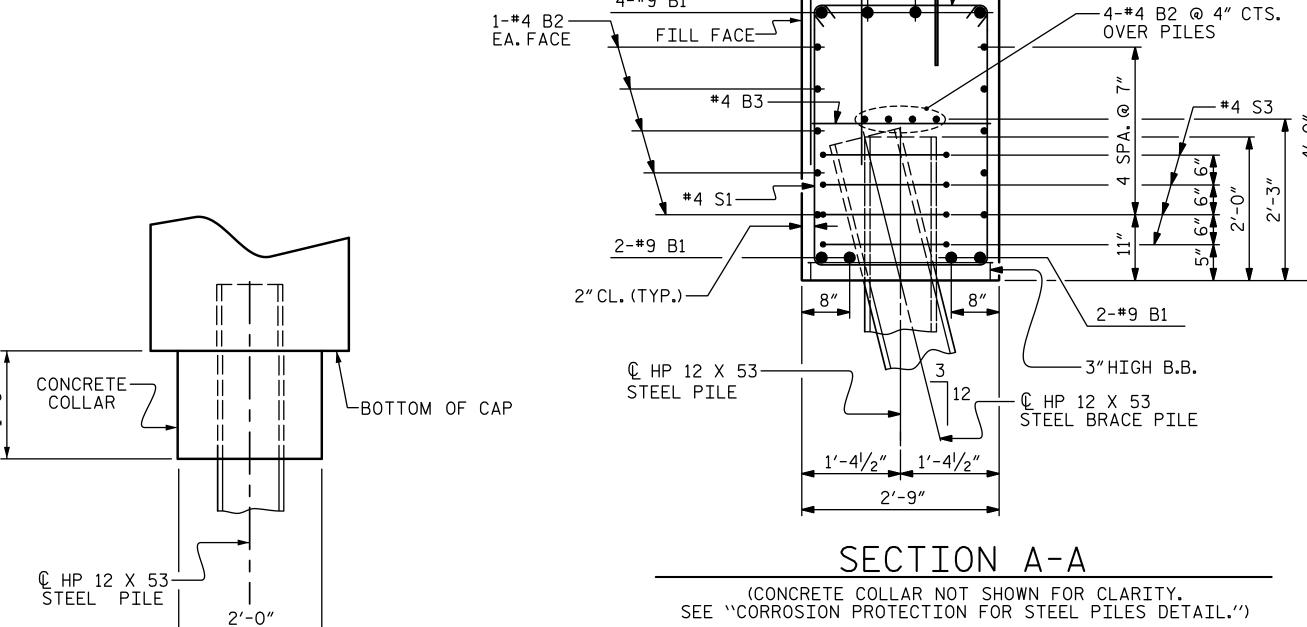
PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES

NO: 7

1'-10" FILL FACE 1" X 9" X 2'-9" —— ELASTOMERIC BRG. PAD (TYPE II, TYP.) BEARING ─  $1'-4\frac{1}{2}''$   $1'-4\frac{1}{2}''$ -#8 D1 DOWELS TO PROJECT 1'-3" ABOVE CAP (TYP.) 2'-9" — ¢ box beam DETAIL "A" FILL FACE



PLAN ELEVATION CORROSION PROTECTION FOR STEEL PILES DETAIL



1'-0"

#4 V2—

CONST. JT.

1'-101/2"

-€ #8 D1 DOWEL

r#4 S2 ←

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B-5817 PROJECT NO.\_ ANSON COUNTY

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SHEET 3 OF 3

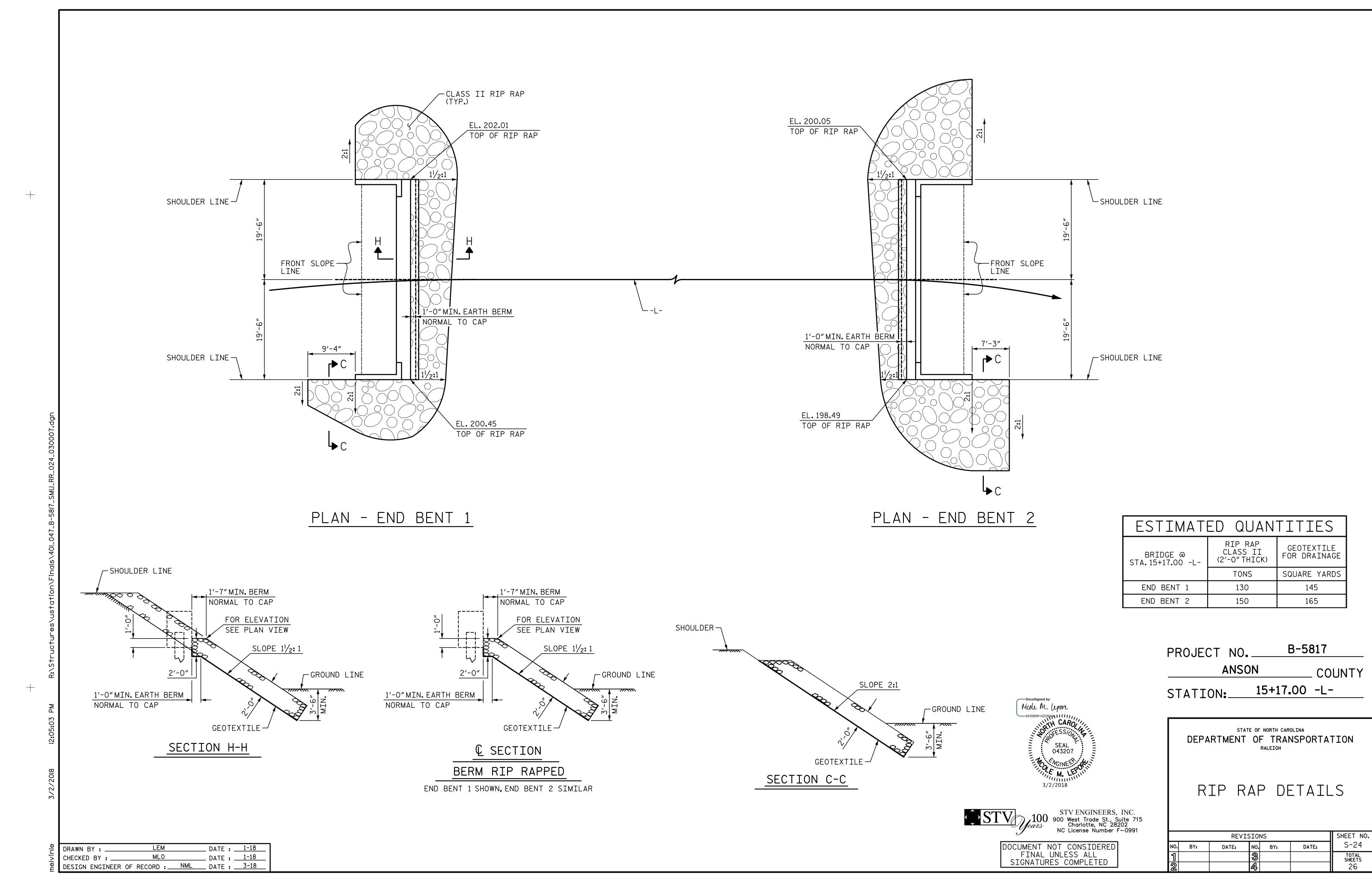
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

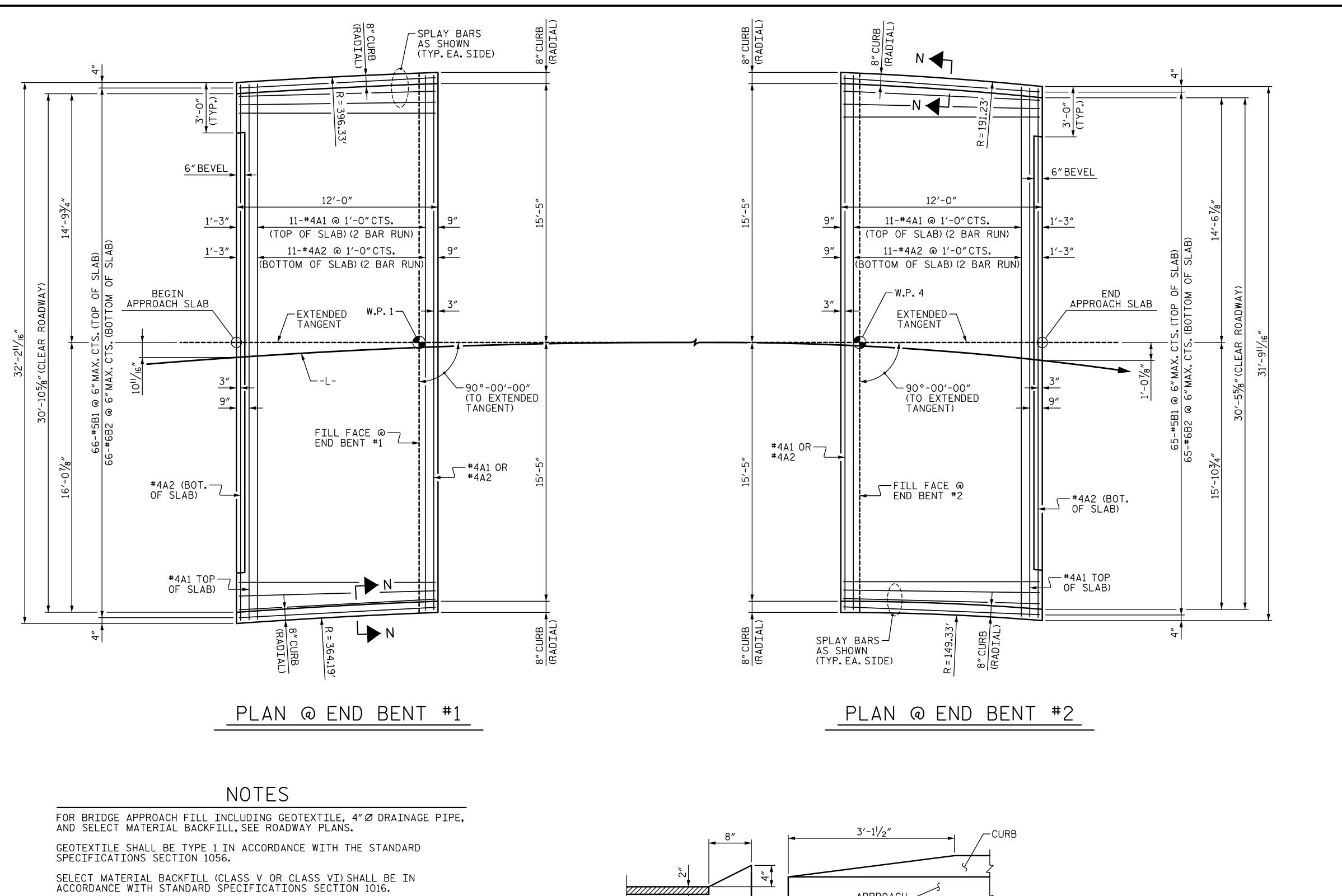
SUBSTRUCTURE

END BENT No. 2 DETAILS

SHEET NO. **REVISIONS** S-23 DATE: DATE: NO. BY: NO. BY: TOTAL SHEETS 26

\_\_ DATE : <u>1-18</u> DRAWN BY : \_\_\_\_\_ DATE : <u>1-18</u> MLO DESIGN ENGINEER OF RECORD : NML DATE : 3-18



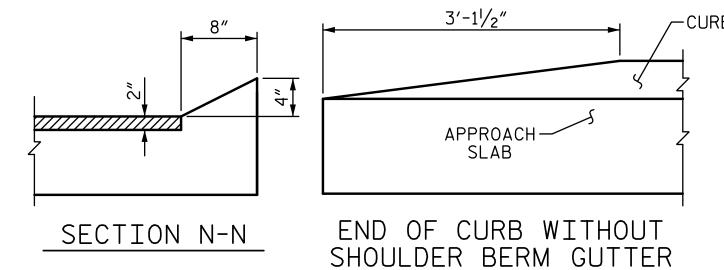


SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

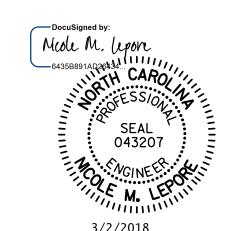
FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.



CURB DETAILS





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PROJECT NO	B-5817
ANSON	COUNTY

BILL OF MATERIAL

APPROACH SLAB AT EB 1

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

APPROACH SLAB AT EB 2

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

291

769

1157

1448

1063

18.4

294

291

757

1139

1430

17.0

LBS.

LBS.

C. Y.

LBS.

LBS.

C. Y.

EPOXY COATED UNCOATED

2'-0"

2'-6"

\* A1 26 #4 STR 16'-11" A2 | 26 | #4 | STR | 16'-9"

\*B1 | 66 | #5 | STR | 11'-2"

\* A1 | 26 | #4 | STR | 16'-11"

A2 26 #4 STR 16'-9"

\*B1 | 65 | #5 | STR | 11'-2"

B2 | 65 | #6 | STR | 11'-8"

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

\* EPOXY COATED

REINFORCING STEEL

**\***EPOXY COATED

B2 | 66 | #6 | STR | 11'-8"

15+17.00 -L-STATION:

SHEET 1 OF 2

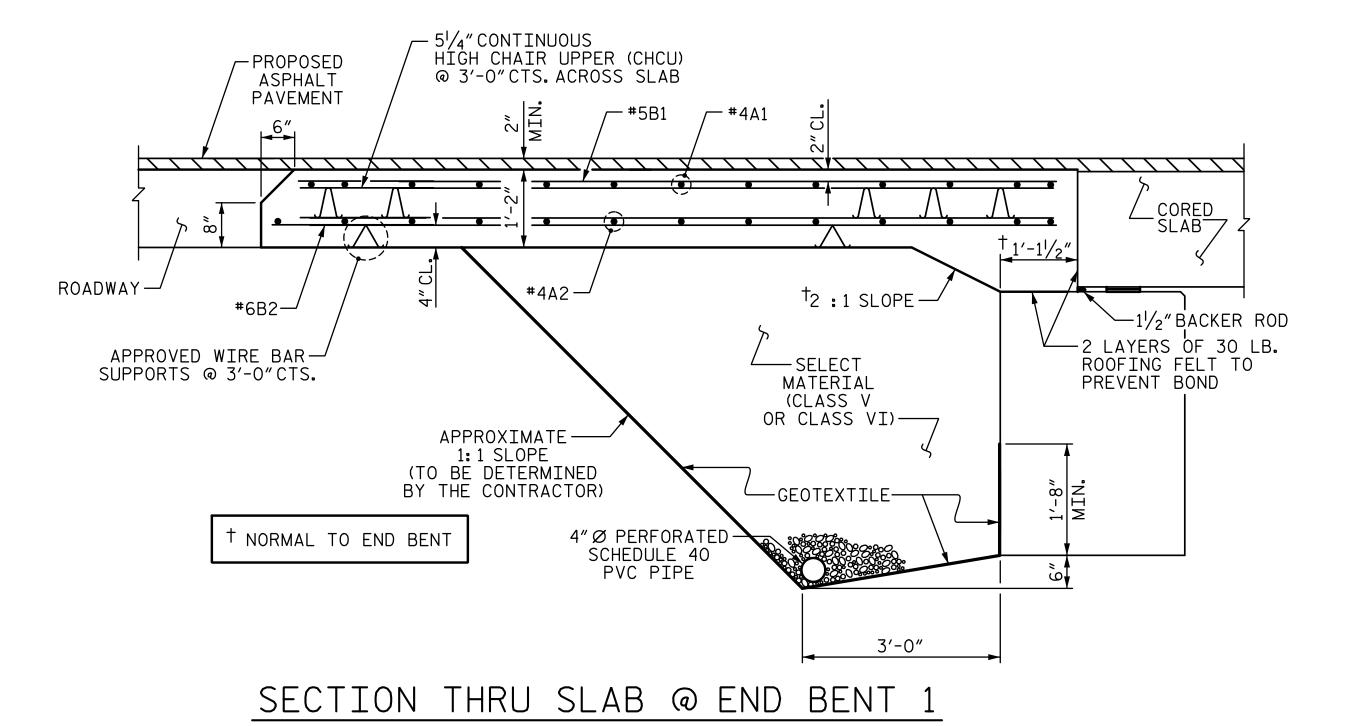
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

BRIDGE APPROACH SLAB (SUB-REGIONAL TIER)

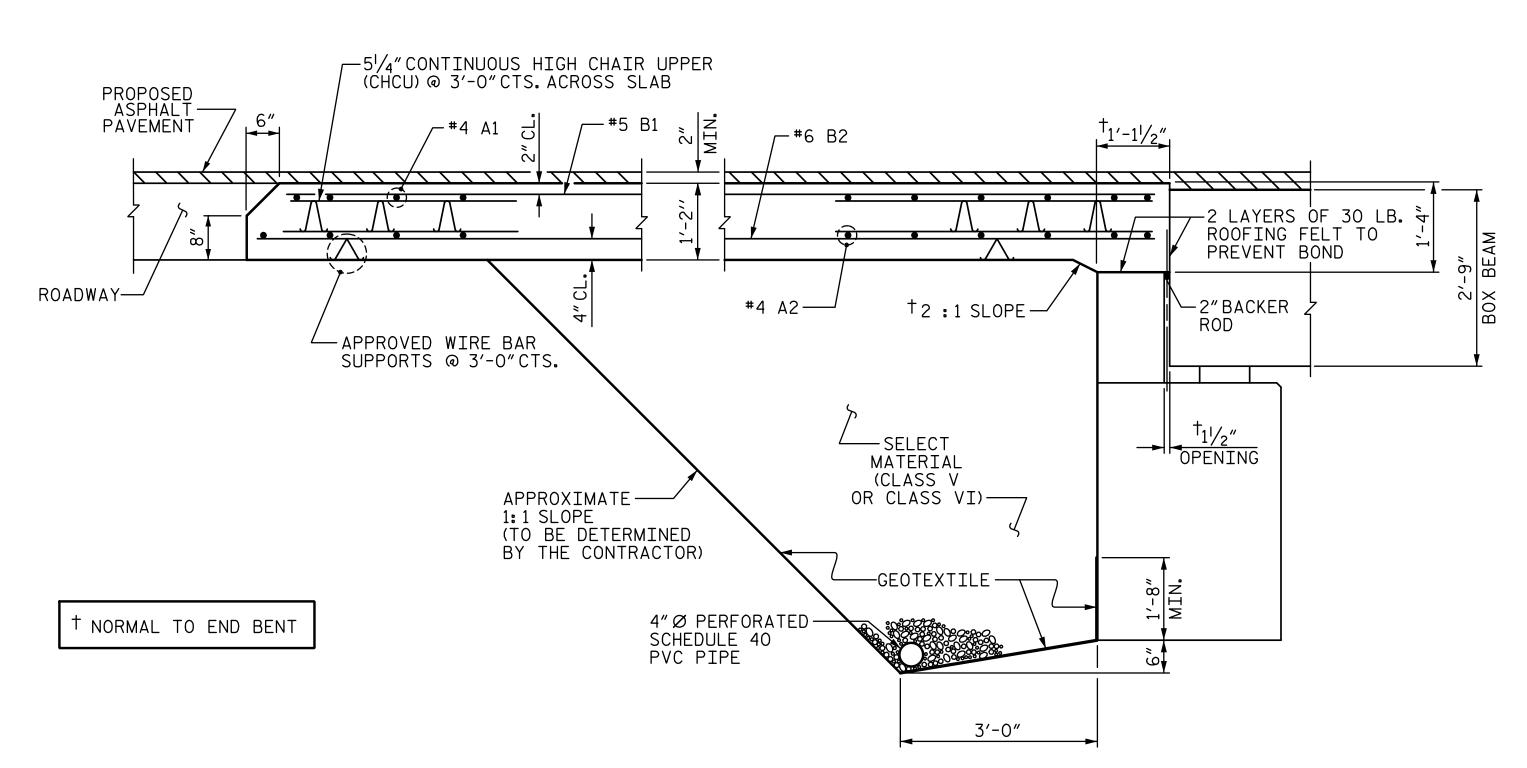
REVISIONS					SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-25
		3			TOTAL SHEETS
		4			26

DRAWN BY : LEM \_\_\_\_\_ DATE : <u>1-18</u> MLO CHECKED BY : \_\_\_\_ DESIGN ENGINEER OF RECORD : NML DATE : 3-18

\_\_ DATE : <u>1-18</u>



(TYPE II - MODIFIED APPROACH FILL)



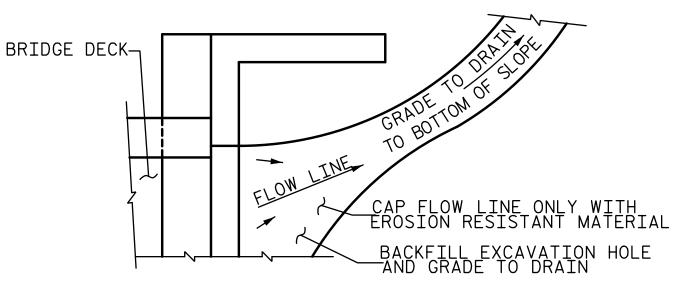
## SECTION THRU SLAB @ END BENT 2

(TYPE II - MODIFIED APPROACH FILL)

DRAWN BY: LEM DATE: 1-18

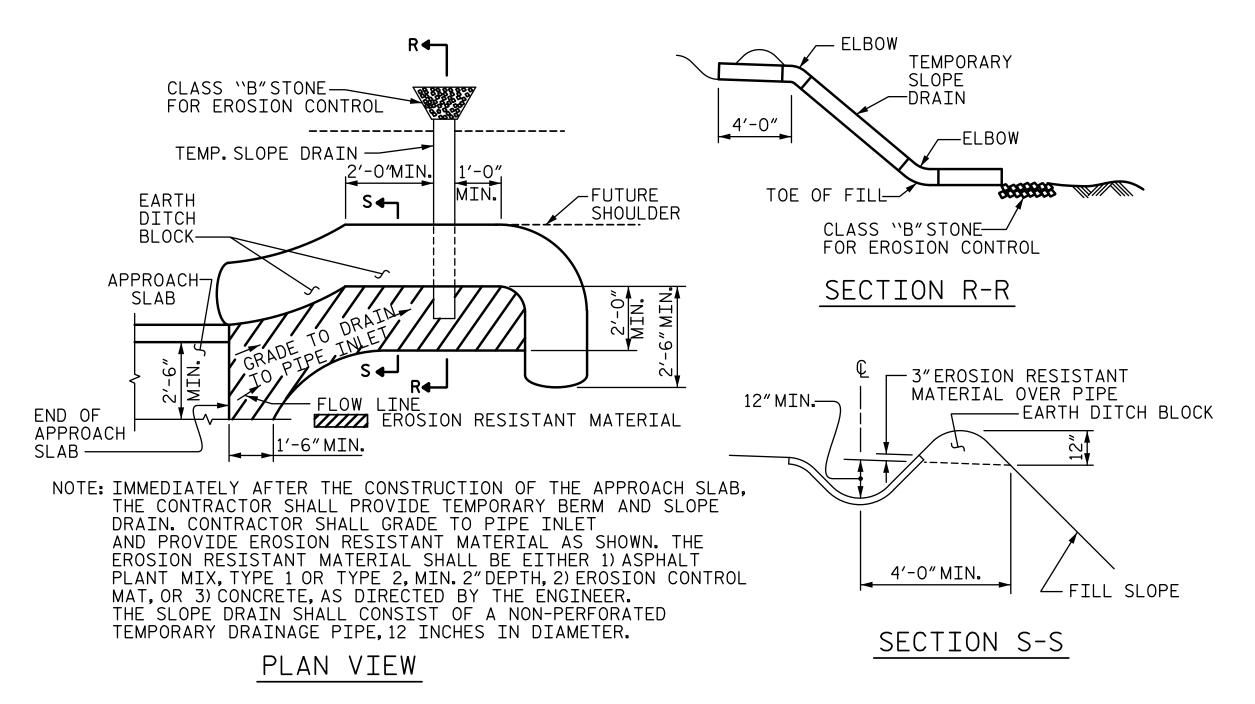
CHECKED BY: MLO DATE: 1-18

DESIGN ENGINEER OF RECORD: NML DATE: 3-18



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

### TEMPORARY DRAINAGE DETAIL



## TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



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SHEET 2 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

BRIDGE APPROACH SLAB DETAILS

(SUB-REGIONAL TIER)

REVISIONS						SHEET NO.
١.	BY:	DATE:	NO.	BY:	DATE:	S-26
1			3			TOTAL SHEETS
?			4			26

## STANDARD NOTES

### DESIGN DATA:

### MATERIAL AND WORKMANSHIP:

EQUIVALENT FLUID PRESSURE OF EARTH

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

---- 30 LBS.PER CU.FT.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

#### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{1}{8}$ " Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 -  $\frac{1}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{1}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 -  $\frac{1}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST \( \frac{1}{6}'' \) IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990